# TransForM - A New Regional Travel Demand Model Developed for Prince George's County 

Project Final Report

Prepared for:
The Maryland-National Capital Park and Planning Commission

June 15, 2006

Prepared by:
Caliper Corporation
1172 Beacon St
Newton, MA 02461.
617-527-4700
www.caliper.com

## CHAPTERS

1. Introduction, Background, and Overview ..... 6
2 TAZ Geography and Highway and Transit Network Development ..... 9
3 Trip Generation. ..... 19
4 Trip Distribution ..... 38
5 Mode Choice. ..... 52
6 Time of Day ..... 68
7 Traffic Assignment. ..... 71

## LIST OF FIGURES

2.1 TAZ Geography ..... 10
2.2 A close-up view of the TransForM Model Network ..... 12
2.3 A close-up view of the MWCOG Network at the same location ..... 13
2.4 Comparison of the two networks at the I-495 and I-295 interchange ..... 14
2.5 Sample Routes in the DC and Prince George's County Region ..... 18
3.1 Productions and Attractions for the entire region. ..... 34
3.2 Total Productions and Attractions for Prince George's County ..... 35
3.3 HBW Productions and Attractions for the entire region. ..... 36
3.4 HBW Productions and Attractions for Prince George's County. ..... 37
4.1 HBW Friction Factors. ..... 42
4.2 HBS Friction Factors ..... 42
4.3 HBO Friction Factors ..... 43
4.4 NHB Friction Factors ..... 43
4.5 HBW Trips: Survey versus Model Trip Frequencies. ..... 45
4.6 HBS Trips: Survey versus Model Trip Frequencies. ..... 45
4.7 HBO Trips: Survey versus Model Trip Frequencies ..... 46
4.8 NHB Trips: Survey versus Model Trip Frequencies ..... 46
4.9 HBW Trip Patterns ..... 48
5.1 Sub-regions for Mode Choice Analysis. ..... 62
5.2 Mode Shares of HBW Trips - Origin ..... 65
5.3 Mode Shares of HBW Trips - Destination ..... 66
6.1 Time of Day Distribution from the COG 2000 Panel Survey. ..... 69
7.1 Volume Delay Functions based on Functional Class ..... 73
7.2 Daily Regional Traffic Flow Map ..... 77
7.3 Daily Traffic Flows: Beltway, DC and Prince George's County ..... 78
7.4 AM Traffic Flows: Beltway, DC and Prince George's County ..... 79
7.5 PM Traffic Flows: Beltway, DC and Prince George's County ..... 80
7.6 Prince George's County Screenlines ..... 88

## LIST OF TABLES

3.1 Trip Production Rates by County ..... 23-26
3.2 Trip Attraction Rates by Trip Purpose and Area Type. ..... 28
3.3 Truck Trip Rates ..... 29
3.4 Non-Motorized HBW Percentages ..... 29
3.5 Internal-External and External-Internal Trip Percentages ..... 30
3.6 Final Productions and Attractions by Trip Purpose ..... 31
3.7 Comparison of MWCOG and PGC TransForM model results ..... 31
3.8 Total Productions by Trip Purpose and Jurisdiction..... ..... 32
3.9 Total Attractions by Trip Purpose and Jurisdiction ..... 33
4.1 Zone Terminal Times based on Employment Density ..... 40
4.2 Transit Percentages by Income Group. ..... 40
4.3 Observed Trip Lengths from the COG 2000 Panel Survey ..... 41
4.4 Trip Distribution Results ..... 47
4.5 HBW Jurisdiction to Jurisdictions Trips ..... 49
4.6 HBS Jurisdiction to Jurisdictions Trips. ..... 49
4.7 HBO Jurisdiction to Jurisdictions Trips. ..... 50
4.8 NHB Jurisdiction to Jurisdictions Trips. ..... 50
4.9 Medium Truck Jurisdiction to Jurisdictions Trips ..... 51
4.10 Heavy Truck Jurisdiction to Jurisdictions Trips. ..... 51
5.1 Utility Specification for the Basic Mode Choice MNL Model ..... 54-56
5.2 Utility Specification for the Carpool MNL Model ..... 56-57
5.3 HBW Mode Choice Shares. ..... 58
5.4 HBS Mode Choice Shares. ..... 58
5.5 HBO Mode Choice Shares ..... 59
5.6 NHB Mode Choice Shares ..... 59
5.7 Estimated versus Observed Mode Choice Shares ..... 60
5.8 HBW Mode Choice Shares by Jurisdiction (Origin) ..... 60
5.9 HBW Mode Choice Shares by Jurisdiction (Destination) ..... 61
5.10 HBW Drive Alone Sub-region to Sub-region Shares ..... 63
5.11 HBW Transit Sub-region to Sub-region Shares ..... 63
5.12 HBW Carpool Sub-region to Sub-region Shares ..... 64
5.13 Final Trips by Mode ..... 67
6.1 Miscellaneous Time of Day Factors ..... 68
6.2 Time of Day Departure and Return Percentages ..... 70
6.3 Time of Day Results ..... 70
7.1 BPR Link Delay Parameters ..... 72
7.2 Regional VMT Estimates ..... 76
7.3 Daily RMSE Statistics ..... 82
7.4 AM RMSE Statistics ..... 83
7.5 PM RMSE Statistics ..... 84
7.6 Off-Peak RMSE Statistics ..... 85
7.7 VMT Comparison on links that have counts ..... 86
7.8 VMT Comparison for Prince George's County ..... 86
7.9 VMT Comparison for other Counties (All Functional Classes). ..... 87
7.10-7.21 Prince George’s County Screenline Tables ..... 89-96

## Chapter 1 Introduction, Background, and Overview

This report documents the model components, model inputs, process and results of the new travel demand model that has been developed for the Prince George's County Planning Department of the Maryland-National Capital Park and Planning Commission (M-NCPPC). The model has been implemented in TransCAD 4.8 and embodies many enhancements to prior models and the MWCOG TPB TP+ Travel Forecasting Model.

The overall objective of this project has been to provide a forecasting tool that will be more suitable for modeling land use impacts and transportation improvements in Prince George's County. The new model adds considerable network and traffic analysis zone detail in Prince George's County, has calibration updated to the year 2000 using the most recent data available, employs some improved modeling procedures, and more closely matches validation data than previous models.

Apart from greater geographic detail and the use of a more recent base year, there were many other model development considerations. One included retaining a similar model structure to make it easier to accommodate future MWCOG updates. A second was the desire to make use of more user-friendly, Windows and GIS-based modeling software as embodied in TransCAD. A third consideration was to employ better algorithms for transit pathfinding, trip distribution, and traffic assignment and to achieve higher convergence and closer calibration of the model to observed data. Lastly, there was an attempt to consider TRB review criticisms of the MWCOG model and address them to the extent possible.

The new Prince George's County Transportation Forecasting Model (TransForM) is a regional model that has the same geographic modeling scope as the MWCOG regional model. The MWCOG model was developed in TP+ and encompasses a 6,800 -squaremile study area. The MWCOG modeled area is currently comprised of 2,191 Traffic Analysis Zones or TAZs and encompasses 22 of the region's major jurisdictions spanning the District of Columbia, Northern Virginia, suburban Maryland and one county in West Virginia.

The other important precursor was a SYSTEM 2 model for Prince George's County that was implemented in 1992 and last updated in 1995. This model had greater geographic detail for Prince George's County than the MWCOG regional model.

The original concept for this project was to convert the regional model from TP+ to TransCAD and add further detail from the System II model and closer calibration in Prince George's County. In the first phase of the project, the original TP+ components were transferred to TransCAD. TransCAD was then run and successfully replicated the 1994 model results fairly closely. During this conversion process, a new interface was implemented and most of the TP+ and FORTRAN programs were converted to TransCAD scripts.

The second phase of the model development effort focused on examining and revising the data and model components used by each of the model steps. Geographically accurate road and transit networks were developed and substituted for the networks used in the MWCOG model. This was a major effort that was performed by Caliper without using project funds so it could be distributed to others. The road network was conflated and realigned so that it closely matches aerial photography for the region and replicates the actual shape of roadway links. Also, major roads were coded with two one-way links and interchanges and access road details were added. The key attributes of the network that affect model computations such as link functional classes, number of lanes and directionality were thoroughly revised and corrected as needed.

A new base year transit network was also developed that is more geographically accurate and is consistent with the new road network. GPS data from WMATA was utilized to obtain the correct route alignments for bus routes. The regional rail network was also created using GIS files.

The model is based on a 2,523 -zone system of which 2,476 zones are internal and 47 are external. There are 885 zones in PGC instead of the 381 original zones in Prince George's County from the MWCOG model.

The model uses the latest demographics from MWCOG. Demographics for the more detailed PGC TAZs were further disaggregated using US Census 2000 data for the region.

The centroid connectors for the entire network were regenerated for the expanded zone system and more geographically accurate road network. These were reviewed in light of network loadings and comparisons with validation data.

A major effort was undertaken to collect and integrate traffic count data in the model network. Traffic counts from many sources were brought in and transferred to the highway network using special GIS tools. Electronic copies of traffic counts were obtained via M-NCPPC staff from the Maryland State Highway Administration, the District of Columbia Department of Transportation, the Virginia Department of Transportation, and the Prince George's County Department of Public Works and Transportation.

The model uses a comprehensive new transit route system that combines both the peak and the offpeak networks in one database, thereby eliminating the need to maintain several versions of the route systems and highway databases. The rail routes were also merged into the same system. Further, the route system is based on the same highway database that is used to run the travel demand models. As a result, improved representations of transit access, transfers, and egress were introduced.

Each component of the four-step model was revised and modified. Some model steps were redeveloped from scratch whereas others were modified to a lesser extent. A complete description of each of the model components is provided in subsequent chapters
of this report. A major difference that is prevalent across all model steps is the elimination of various adjustments factors used by the COG model.

Significantly more computation is performed, resulting in more highly converged model steps for trip distribution and trip assignment. In addition, the model employs a convergent feedback loop procedure.

The model makes extensive use of a Year 2000 wave of a panel survey conducted by the MWCOG. Despite the relatively small sample size, we found the data to be of high quality and sufficient to build and calibrate the trip generation and trip distribution models that were developed.

The third phase of the project involved calibration and refinement of the model based upon comparisons with external data that was used for validation. The new model matches validation targets much more closely than the MWCOG model throughout the region and matches ground counts especially well in Prince George's County and Montgomery County.

A user-friendly interface was developed to make it easy to perform model runs. The interface provides a push-button means of selecting scenarios and launching model runs. It also makes it extremely easy to produce informative graphics illustrating model results.

The remainder of this report provides a detailed description of the data preparation, model estimation, model application, and validation. A companion User's Guide explains how to use the model software.

## Chapter 2 TAZ Geography and Highway and Transit Network Development

This chapter describes the development of the key databases used in the PGC TransForM model. These include the TAZ database and the highway and transit networks.

The TAZ database was developed by the Maryland-Capital Planning Park and Planning Commission and is an expanded version of the MWCOG TAZ database with a much larger number of zones in Prince George's County. While for consistency, the TransForM model uses the latest round of demographics from the MWCOG, these data needed to be disaggregated for the smaller zones in PGC.

This chapter also describes the development of the highway and transit networks that are used in the PGC TransForM model. New networks using a variety of GIS data and other sources were developed. The networks have much greater geographic accuracy than those used in prior modeling efforts and considerable effort was also expended to correct attribute information as well. This work was done by Caliper outside of the contracted effort so that the road network could be made available to other TransCAD users.

A key aspect of the road network development effort was the addition of considerable additional detail in Prince George's County. The resulting network has all of the links that were in the earlier System II network for Prince George's County and all of the links elsewhere in the region that are in the MWCOG model. In addition, to improving the road geography, divided highways are explicitly represented and ramps were corrected and added as appropriate.

The new regional network was aligned over aerial photography throughout the entire region. This results in a very accurate network for planning and also served as the basis for the development of a new transit network. Bus routes in the transit network sit directly on the roads in the highway network. When necessary, links were added to the road network to make this possible. Rail links were also added to represent WMATA rail and commuter rail services. Further, a single comprehensive transit network consisting of both the peak and the off-peak routes was developed. The following sections describe the methods and procedures used to develop these databases.

## TAZ Database Preparation

The model uses a TAZ database consisting of 2,523 zones out of which 47 are external stations. This database was developed from the MWCOG model in which the zones in Prince George's County were expanded. The number of zones in Prince George's county was increased from 381 in the MWCOG model to 885 in the new database. The updated zone geography for Prince George's County was provided by the Maryland-Capital Park and Planning Commission. The zone geography in the new model is the same as the previous system for all zones outside Prince Georges' County.

Figure 2.1 shows the TAZ geography for the region.


Figure 2.1 TAZ Geography

## TAZ Demographics

The TAZ demographics consist of households, population and employment estimates by industry. These were generated from the 2000 COG 7.0 Cooperative Forecasts. However, since the COG 2000 Forecasts were based on the MWCOG zonal system, the demographics had to be disaggregated for zones in Prince George's County. In order to disaggregate the demographics, we first generated the demographics for the newer zones in Prince George's County using Census 2000 data. For a large zone in the MWCOG model, the COG 7.0 forecasts were disaggregated to the newer zones using the ratios of these Census 2000 demographics estimates.

## Highway Network Preparation

The model uses a conflated and realigned highway network that incorporates all of the links that were present in the previous model. The work was done in close conjunction with the Maryland Park and Planning Commissions (MNCPPC). There were several tasks performed in highway network preparation.

First, aerial photography and GIS datasets were assembled from various sources. Next, the SYSTEM II network from Prince Georges' County and the MWCOG network were imported into TransCAD. The imported networks were merged by a process that deleted the PGC links from the MWCOG network and replaced them with those from SYSTEM II. The resulting network was then virtually completely redone as it was then conflated and realigned to sit directly on top of the aerial imagery.

Conflation is a process in which links in the network line layer are replaced by links that have more accurate geography. Typically, this means they have many more "shape" and also more accurately place shape points. However, it also involves correcting the beginning and ending locations of the nodes for links. Links may also be realigned or reshaped directly on top of high resolution aerial photography. This may entail a similar process of adding or correcting the location of beginning and ending nodes as well as shape points. Conflation and realignment can be used together to achieve the best results. The conflation and realignment effort resulted in network links that closely match the correct geographic shape of the roads included in the network. During this process, new links were also added such as freeway ramps that did not previously exist in the network. Particular attention was paid to the freeway and freeway/ramp interchanges, "dualizing" link segments and identifying HOV facilities. During the network development procedure, links with incorrect attributes were also identified and the attributes were corrected. PGC staff assisted Caliper in thoroughly reviewing and correcting key attributes in the network such as the link functional class, the link direction flag, number of lanes and link limit codes used to identify HOV facilities and used to designate link prohibitions.

A visual comparison of the MWCOG and new networks illustrates the types of changes that were made and the resultant improvements. Figure 2.2 shows a close-up of the new network at the north-east corner of the beltway and Figure 2.3 shows the same close-up in the MWCOG network. As can be visually seen, the new network is re-aligned and represents actual road geometry. Further, the new network has more local roads than the MWCOG network.

A close-up of the I-295 and I-495 interchange (in both the networks) at the Greenbelt region is shown in Figure 2.4.


Figure 2.2 A close-up view of the TransForM Model Network


Figure 2.3 A close-up view of the MWCOG Network at the same location


Figure 2.4 Comparison of the two networks at the I-495 and I-295 interchange
The benefits are not merely visual however. More accurate measures of network distances and travel times result.

## Centroid Connectors

Before the network could be used, it was necessary to generate new centroid connectors. These were generated using an automated tool that is part of TransCAD. The tool makes it possible to specify rules for generating centroid connectors.

During the first pass with the tool, up to six connectors were permitted for each internal centroid. In this pass, the connectors were prevented from connecting to freeways, expressways, major arterials and ramps and were constrained to lie within the zonal boundaries. The rationale for having multiple connectors is to properly represent travel in all directions between zones.

The connectors generated were reviewed and in some cases additional connectors were added if there were two few that met the criteria specified. Some connectors were also removed if they were redundant. Aerial photography was used in judging the need for additional connectors. Preliminary traffic assignments were also used to judge if the network was being loaded properly.

The connectors for the external zones were initially the same as the ones used in the MWCOG TP+ model. However, in certain areas such as the near Baltimore, the external connectors were modified.

## Integration of Traffic Counts

Traffic counts were obtained from many sources and were transferred to the network.
The traffic counts utilized are those listed below:

- Beltway counts for the DC region
- Virginia Traffic Counts
- Maryland Traffic Counts
- Counts from MNCPPC
- Counts from the ICC study

The raw traffic count information from each of these sources was first converted to a point database. All the counts that pertained to the spring months of 2000 and 2001 were employed. These raw counts were aggregated based on the time period to generate AM. PM and off-peak counts. The counts were then transferred to appropriate links in the network using specialized procedures written in TransCAD for this purpose. Finally, the counts were compared against capacities and any links where the counts were far greater than the capacities were identified and checked.

## Transit Network Preparation

The TransForM model employs a comprehensive route system that incorporates both peak and off-peak transit routes. The appropriate subset is used in building the peak and off-peak transit networks.

There were five major datasets used in the development of the transit networks. The first of these was the highway network described above. The second was a geographically accurate GPS point tracking database that, for every bus route for every minute, tracks the route location. The database was provided by the Washington Metropolitan Area Transit Authority (WMATA). For rail routes, we used Caliper's 1:200,000 scale railroad network. We also utilized the TIGER/LINE Street Network and the MWCOG TP+ peak and offpeak transit networks.

The procedure to develop the transit network had five major steps which are described below.

## Step 1 - Creating Bus Transit Routes from GPS Database for WMATA Area

In the first step, an automated procedure was written to convert the ordered GPS route points from the WMATA tracking database into a TransCAD route system. The route system was based on the TransForM Highway Planning Network. During the conversion, we found that many of the GPS points went over streets that were not in the TransForM network. A procedure was written to automatically identify these streets, using the TIGER/LINE street network, and then add them to the PGC highway network. The route creation procedure was run again and the result was a geographically accurate transit network for the WMATA area. The routes and stops were then checked one-by-one manually to ensure the fidelity and accuracy of the conversion process. Streets were edited and routes were manually realigned as necessary.

## Step 2 - Creating Bus Transit Routes Outside of the WMATA Area from TP+ Transit Networks

Unfortunately, the GPS data points did not encompass the entire study area of the PGC model; thus routes had to be generated outside of the WMATA area using an alternate methodology. The transit routes defined in the MWCOG TP+ model did encompass the entire study area so a procedure was written to convert these TP+ routes into TransCAD route system format. For bus routes wholly inside of the WMATA area, the GPS points were used since the geographic accuracy of this dataset was vastly superior to that of the TP+ routes. The TP+ routes were only used to define transit routes either wholly outside of the WMATA area or crossing into the WMATA area. In the original TP+ dataset, peak and offpeak routes were coded as separate datasets. During the TransCAD conversion, peak and offpeak routes were combined into a single dataset. After
the routes were converted into TransCAD format, each route was checked one-by-one manually to ensure accuracy. Sometimes bad route paths were the result of highway network errors which lead to improvements in the underlying planning network. Due to the crude geography of the original TP+ transit networks, much more extensive route checking and manual editing was necessary.

## Step 3 - Assigning Bus Transit Route Attributes

In this step, we assigned transit route model attributes such as peak and offpeak headways, and peak and offpeak running times. For the TP+ transit routes outside of the WMATA area that were converted, the original TP+ model route attributes were used. For the GPS-converted routes inside the WMATA area, no route attributes existed in the GPS dataset. In order to assign attribute data, each GPS route was manually matched with a corresponding TP+ route based on the similarity of their route names and similarity of route alignment. Then, after matching, the appropriate TP+ route attributes were transferred.

## Step 4 - Creating Fixed Rail (Subway and Commuter Rail) Routes

Since fixed rail routes do not generally go over the street network, a separate fixed rail line database had to be developed. The geography for this line database came from Caliper's 1:200,000 scale nationwide rail network. First, the regional study area portion of this rail network was extracted. Second, unnecessary links such as siding links and track links that were not used by fixed rail roads were taken out. Third, using the TP+ coded fixed rail routes as a guide, fixed rail routes and stops were manually coded in on top of the fixed rail network, and the TP+ route attributes were transferred over. Fourth, the fixed rail routes and fixed rail links were merged into the bus routes and highway planning network. Lastly, connector links were created to provide connectivity between the highway network and the rail network.

## Step 5 - Creation of Transit Networks from Transit Route System

In this step, a TransCAD transit network were created from the converted route system so that the PGC model could create transit skim matrices to feed into the mode split model, and so that the PGC model can perform a transit assignment to estimate transit ridership. First, we adjusted the In-Vehicle Travel Times (IVTT) for the bus routes to match the run times from WMATA. Second, the walk access links were identified in the highway network and filled with a walking time based on speeds of 3 miles per hour. Drive access links were then identified and peak and off peak drive times were computed using the model congested highway times. It should be noted that the walk and drive connectors in the TP+ networks are not used, since the transit networks in the TransForM model use the local streets as access links. For skimming methodology, we used TransCAD's Pathfinder method since it has the highest flexibility in terms of network settings and consistently produces the most reasonable paths compared to alternate methods such as those in TP+ or TRANPLAN.

Figure 2.5 illustrates some of the transit routes


Figure 2.5 Sample Routes in the DC and Prince George's County Region

## Chapter 3 Trip Generation

This Chapter describes the development of the trip generation models that were implemented. Because of the goal of creating a model with a Year 2000 base, the trip generation model was developed utilizing data collected in the COG Panel Survey. We began with an approach that was similar to the MWCOG Version \#2.1D trip generation model but ultimately modified it for trip production modeling while retaining the trip attraction models. For most purposes, trips are balanced to productions as the last part of the trip generation model. This chapter provides background information on the MWCOG trip generation model, discusses the reasons for developing a new trip production model, and describes the PGC TransForM trip generation model development and application processes.

## The MWCOG Trip Generation Model

The trip purposes used in the MWCOG model are:

- Home Based Work Trips (HBW)
- Home Based Shop Trips (HBS)
- Home Based Other Trips (HBO)
- Non Home Based Trips (NHB)
- Medium Truck Trips (single unit, 2 Axles, 6 or more tires)
- Heavy Truck Trips (all combination vehicles)
- Internal External Trips (IX)
- External Internal Trips (XI)

The trip production in the MWCOG model is performed based on the three dimensions of Income (with 4 categories approximating income quarterlies), Household Size (with 1, 2, 3 and 4 and over persons per household) and the number of vehicles in the household (with 0, 1, 2 and 3+ vehicles). The trip rates were determined from the COG/TPB 1994 Household Travel Survey and applied to the entire region. However, these trips were first adjusted by a global factor to account for underreporting of trips in the survey data and were then subsequently adjusted at the jurisdiction level using jurisdiction factors that varied by trip purpose.

The trip attraction models were regression equations derived from the 1994 survey and primarily used employment data. The computed attractions were then multiplied by global factors by purpose to account for underreporting of trips. Subsequently, the attractions were multiplied by factors that varied by district.
The truck trips were obtained using regression equations derived from the 1996 Truck Internal and External Survey.

For the HBW trip purpose, the non-motorized trip productions were subtracted out using walking percentages based on area types. The non motorized attractions were determined using a regression equation and subtracted out.

The productions obtained by the cross-classification also included Internal External (IX) and External Internal (XI) trips. These were subtracted from the productions. The share of Internal External (IX) productions were computed using a model as below: IXP $=0.079 * \operatorname{Exp}(-0.088 * \mathrm{DNE})$, where IXP denotes the percentage of IX productions for the zone and DNE denotes the distance to the nearest zone.

Further details of the MWCOG trip generation process are provided in the COG/TPB Travel Forecasting Model, Version 2.1D Draft \#50 Calibration report.

## Trip Generation Analysis

The basic approach taken was to attempt to use the Year 2000 Wave 3 of the COG 19982002 Washington Region Longitudinal Household Travel Surveys as the source for the trip generation models. The panel survey had a sample size of approximately 2400 households. Despite the rather small sample size, we felt that the more recent data might provide a superior model and that this might avoid the use of a large number of correction factors.

The 2000 Panel survey data were imported into TransCAD and were processed to yield a file of the number of trips by trip purpose for each survey respondent. To do this, the trips were first classified into the trip purpose categories. Then the number of trips in each category was tabulated. Lastly, the household and individual characteristics were appended to the person level trip file. This resulted in an aggregated file that contained one record for every person and contained the number of trips (of each trip purpose) made by the person in addition to the characteristics of the person and the household. These data were used to determine trip generation determinants and rates.

Statistical analysis of trip rates tabulated from the survey was the primary basis for developing the market segmentation that was used. A wide variety of different explanatory factors were investigated. Early on in the analysis, we decided to model person trip rates rather than household trip rates. This has several advantages an important one being that it increases the sample size considerably. It also appears to increase the variation in travel behavior to be explained. In other words, aggregating to the household level lessens the variation in trip rates. Lastly, on theoretical grounds it provides a richer theory for how households share certain trip making responsibilities. For example, in households with two adults and only one worker, the non-worker tends to make a greater number of shopping trips.

Another element of the trip generation analysis was an attempt to distinguish trip rates by tripmakers' geographic location within the region. This was done by tabulating separate trip generation rates for the District of Columbia and each jurisdiction covered in the survey in Maryland and Virginia. The jurisdictions in Maryland were the counties of Prince Georges, Montgomery, Calvert, Charles, and Carroll and Frederick Counties combined. The jurisdictions in Virginia were the independent cities of Arlington, Alexandria, the combination of Falls Church County, Fairfax County and Fairfax City, Loudon County, Manassas and Prince William Counties, and Spotsylvania and Stafford

Counties. For other areas in the model for which trip rates could not be determined from the survey either due to unavailable records or sample size issues, we used the trip rates from adjacent areas.

The statistical tools utilized were multiple regression and binary logit analysis. Preliminary regressions established that there was significant variation in trip rates by geography and that different market segment categories might work better than using the same exact dimensions used in the MWCOG model. Because for many people, only zero or one trip were reported for a particular trip purpose, we switched to logit analysis to identify the key classifications that we would use.

In order to determine the variables that affect the trip making patterns for the HBW, HBS, HBO and NHB trip purpose, binary logit models were estimated for each trip purpose. The binary logit model was used to determine the factors that were associated with a person making a trip. Thus, records in the survey database were flagged to identify whether the person made a trip of for each particular trip purpose. The binary field that represents whether a person made a trip (of a particular purpose) was used as the dependent choice variable for the binary logit estimation procedure.

Based on the household and person characteristics, several variables were generated (such as a dummy variable representing whether the household in which the person has access to a car or not, whether the person is a worker or not etc). For each trip purpose, several binary logit models were estimated for each county as well as for the entire region. A broader set of key variables were examined during the process including household size, auto availability, worker status, household income, number of children in the household, the age and the sex of the person. Some of these variables such as the last three were not significant when other factors were taken into account. After several tests, the best model was determined for each trip purpose and the trip production classification was identified. Based upon the analysis, we picked the dimensions of worker status, the number of persons 16 and over in the household, and vehicle ownership. These dimensions worked well across trip purposes and were used to tabulate the survey responses for the geographical segmentation described above. Due to the limited sample, we restricted the number of variables and classification levels to a modest number. It should be noted that with a larger sample, a more refined set of models could be developed. Also, more elaborate models could be constructed including models of tour generation.

An example of the analysis performed follows. Shown below are the estimation results for the best model for the probability of a person making a home-based shopping (HBS) trip purpose.

| Parameter | Estimate | Std. Error | T Test |
| :--- | :---: | :---: | :---: |
| ----------------------------------------------------------------- |  |  |  |
| CONSTANT | -1.853497 | 0.089261 | -20.764883 |
| HHSize1GE16 | 0.496484 | 0.104321 | 4.759212 |
| HHSize2GE16 | 0.244135 | 0.076157 | 3.205703 |
| AUTOAVAIL | 1.053245 | 0.091283 | 11.538193 |
| WORKER | -0.612797 | 0.075291 | -8.139072 |

Maximum likelihood reached after 10 iterations.

| Source | Df | SS | Mean SS | F Ratio |
| :---: | :---: | :---: | :---: | :---: |
| Model | 4 | 432.50043 | 108.12511 | 633.97608 |
| Error | 5428 | 925.74957 | 0.17055 |  |
| Total | 5432 | 1358.25000 |  |  |

$$
\begin{aligned}
& \text { R Squared }=0.318425, \text { Adjusted R Squared }=0.317922 \\
& \text { LL(0) }=-3765.87, \text { LL }(c)=-2925.54 \\
& \text { LL(Bhat })=-2835.36 \\
& -2[\operatorname{LL}(0)-\operatorname{LL}(\text { Bhat })]=1861.02 \\
& -2[\text { LL(c) }) \text { LL(Bhat) }]=180.374 \\
& \text { Rho Squared }=0.247091, \text { Adjusted Rho Squared }=0.245763 \\
& \text { Root Mean Square Error }=0.412978
\end{aligned}
$$

The above model results illustrate that for the HBS trip purpose, the probability of making a trip depends significantly upon 3 variables, the household size (whether the household has 1 person over the age of 16 , the auto availability of the household and whether the person is a worker or not. Note that workers are less likely to make shopping trips than non-workers. In particular, the sign of the parameters is consistent with expectations and the $t$ test results indicate that these variables are significant.

It was found that the above sparse set of variables were significant for the other trip purposes as well. The estimation was also performed on subsets of the survey data by county of residence for each survey respondent. In this analysis, it was found that while the same variables were significant, the coefficients of the estimates varied significantly by location. This was also later seen from the computation of the trip rates for each county based on the above classification.

## TransForM Trip Production Rates

The aggregated trip survey file was used to determine trip rates by county of residence Region wide trip rates based on the county of residence were generated from the survey for the above classification. If the trip rates of two or more counties were very similar,
they were grouped into one classification. Whenever accurate estimates of trip rates for a particular county and classification could not be generated due to sample size restrictions, the region average or an alternative county's rates were used.

For the HBW trip purpose, any person who is not a worker is deemed to make no HBW trips. The trip rates are listed for the following jurisdictions.

- District of Columbia
- Calvert County, Maryland
- Charles County, Maryland
- Carroll and Frederick Counties, Maryland
- Montgomery County, Maryland
- Prince Georges County, Maryland
- Arlington, Virginia
- Alexandria, Virginia
- Falls Church County, Fairfax County and Fairfax City, Virginia
- Loudoun County, Virginia
- Manassas and Prince William Counties, Virginia
- Spotsylvania and Stafford Counties, Virginia

The following table highlights the trip production rates for each of the 4 basic purposes:

| County | Auto <br> Avail | Worker | lersons <br> $16+$ | HBWRate | HBSRate | HBORate | NHBRate |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| DC | 0 | 0 | 1 | 0.00 | 0.63 | 0.91 | 0.42 |
| DC | 0 | 0 | 2 | 0.00 | 0.30 | 0.70 | 0.23 |
| DC | 0 | 0 | 3 | 0.00 | 0.30 | 0.87 | 0.40 |
| DC | 0 | 1 | 1 | 1.28 | 0.29 | 0.31 | 0.87 |
| DC | 0 | 1 | 2 | 1.24 | 0.26 | 0.47 | 0.60 |
| DC | 0 | 1 | 3 | 1.35 | 0.22 | 0.48 | 0.37 |
| DC | 1 | 0 | 1 | 0.00 | 1.13 | 1.96 | 0.45 |
| DC | 1 | 0 | 2 | 0.00 | 0.60 | 1.64 | 0.51 |
| DC | 1 | 1 | 3 | 0.00 | 0.34 | 1.29 | 0.54 |
| DC | 1 | 1 | 1 | 1.35 | 0.39 | 0.93 | 1.05 |
| DC | 1 | 1 | 2 | 1.22 | 0.28 | 1.11 | 1.04 |
| DC | 0 | 0 | 1 | 1.14 | 0.47 | 1.25 | 1.04 |
| CALVERT_MD | 0 | 0 | 2 | 0.00 | 0.18 | 1.06 | 1.02 |
| CALVERT_MD | 0 | 0 | 3 | 0.00 | 0.18 | 1.06 | 1.02 |
| CALVERT_MD | 0 | 1 | 1 | 1.19 | 0.18 | 1.06 | 1.02 |
| CALVERT_MD | 0 | 1 | 2 | 1.19 | 0.18 | 1.06 | 1.06 |
| CALVERT_MD | 0 | 1 | 3 | 1.19 | 0.18 | 1.06 | 1.02 |
| CALVERT_MD | 1 | 0 | 1 | 0.00 | 0.18 | 1.06 | 1.02 |
| CALVERT_MD | 1 | 0 | 2 | 0.00 | 0.26 | 1.06 | 1.02 |
| CALVERT_MD | 1 | 0 | 3 | 0.00 | 0.18 | 1.06 | 1.02 |
| CALVERT_MD | 1 | 0 | 1 | 1.19 | 0.18 | 1.06 | 1.02 |
| CALVERT_MD | 1 | 1 | 1 | 1.06 |  |  |  |
| CALVERT_MD | 1 | 1 | 2 | 1.15 | 0.10 | 1.42 | 1.38 |
| CALVERT_MD | 1 | 1 | 3 | 1.43 | 0.18 | 1.06 | 0.36 |


| CHARLES_MD | 0 | 0 | 1 | 0.00 | 0.24 | 1.11 | 0.99 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CHARLES_MD | 0 | 0 | 2 | 0.00 | 0.24 | 1.11 | 0.99 |
| CHARLES_MD | 0 | 0 | 3 | 0.00 | 0.24 | 1.11 | 0.99 |
| CHARLES_MD | 0 | 1 | 1 | 1.14 | 0.24 | 1.11 | 0.99 |
| CHARLES_MD | 0 | 1 | 2 | 1.14 | 0.24 | 1.11 | 0.99 |
| CHARLES MD | 0 | 1 | 3 | 1.14 | 0.24 | 1.11 | 0.99 |
| CHARLES_MD | 1 | 0 | 1 | 0.00 | 0.24 | 1.11 | 0.99 |
| CHARLES_MD | 1 | 0 | 2 | 0.00 | 0.60 | 1.11 | 0.99 |
| CHARLES_MD | 1 | 0 | 3 | 0.00 | 0.37 | 1.11 | 0.99 |
| CHARLES_MD | 1 | 1 | 1 | 1.00 | 0.24 | 1.11 | 0.99 |
| CHARLES_MD | 1 | 1 | 2 | 1.07 | 0.20 | 1.09 | 1.09 |
| CHARLES_MD | 1 | 1 | 3 | 1.22 | 0.22 | 1.29 | 1.00 |
| CARR_FRED_MD | 0 | 0 | 1 | 0.00 | 0.35 | 0.94 | 0.99 |
| CARR_FRED_MD | 0 | 0 | 2 | 0.00 | 0.35 | 0.94 | 0.99 |
| CARR_FRED_MD | 0 | 0 | 3 | 0.00 | 0.35 | 0.94 | 0.99 |
| CARR_FRED_MD | 0 | 1 | 1 | 1.23 | 0.35 | 0.94 | 0.99 |
| CARR_FRED_MD | 0 | 1 | 2 | 1.23 | 0.35 | 0.94 | 0.99 |
| CARR_FRED_MD | 0 | 1 | 3 | 1.23 | 0.35 | 0.94 | 0.99 |
| CARR_FRED_MD | 1 | 0 | 1 | 0.00 | 0.35 | 0.94 | 0.99 |
| CARR_FRED_MD | 1 | 0 | 2 | 0.00 | 0.90 | 1.64 | 0.90 |
| CARR_FRED_MD | 1 | 0 | 3 | 0.00 | 0.44 | 1.17 | 0.86 |
| CARR_FRED_MD | 1 | 1 | 1 | 1.23 | 0.35 | 0.94 | 0.99 |
| CARR_FRED_MD | 1 | 1 | 2 | 1.34 | 0.42 | 0.78 | 0.99 |
| CARR_FRED_MD | 1 | 1 | 3 | 1.10 | 0.25 | 1.14 | 0.85 |
| MONTG_MD | 0 | 0 | 1 | 0.00 | 0.41 | 1.14 | 0.85 |
| MONTG_MD | 0 | 0 | 2 | 0.00 | 0.41 | 1.14 | 0.86 |
| MONTG_MD | 0 | 0 | 3 | 0.00 | 0.41 | 1.09 | 0.86 |
| MONTG_MD | 0 | 1 | 1 | 1.25 | 0.41 | 1.14 | 0.85 |
| MONTG_MD | 0 | 1 | 2 | 1.25 | 0.41 | 1.14 | 0.85 |
| MONTG_MD | 0 | 1 | 3 | 1.25 | 0.41 | 1.14 | 0.86 |
| MONTG_MD | 1 | 0 | 1 | 0.00 | 0.41 | 1.83 | 0.86 |
| MONTG_MD | 1 | 0 | 2 | 0.00 | 1.03 | 1.90 | 0.68 |
| MONTG_MD | 1 | 0 | 3 | 0.00 | 0.68 | 1.19 | 0.57 |
| MONTG_MD | 1 | 1 | 1 | 1.31 | 0.59 | 1.06 | 1.06 |
| MONTG_MD | 1 | 1 | 2 | 1.22 | 0.44 | 1.22 | 0.93 |
| MONTG_MD | 1 | 1 | 3 | 1.31 | 0.32 | 1.10 | 0.82 |
| PRINCEG_MD | 0 | 0 | 1 | 0.00 | 0.33 | 0.88 | 0.89 |
| PRINCEG_MD | 0 | 0 | 2 | 0.00 | 0.42 | 0.67 | 0.21 |
| PRINCEG_MD | 0 | 0 | 3 | 0.00 | 0.21 | 1.03 | 0.27 |
| PRINCEG_MD | 0 | 1 | 1 | 1.24 | 0.33 | 0.88 | 0.89 |
| PRINCEG_MD | 0 | 1 | 2 | 1.24 | 0.33 | 0.88 | 0.89 |
| PRINCEG_MD | 0 | 1 | 3 | 1.54 | 0.15 | 0.54 | 0.31 |
| PRINCEG_MD | 1 | 0 | 1 | 0.00 | 0.74 | 1.97 | 0.94 |
| PRINCEG_MD | 1 | 0 | 2 | 0.00 | 0.73 | 1.68 | 0.93 |
| PRINCEG_MD | 1 | 0 | 3 | 0.00 | 0.47 | 1.40 | 0.30 |
| PRINCEG_MD | 1 | 1 | 1 | 1.26 | 0.39 | 0.86 | 1.27 |
| PRINCEG_MD | 1 | 1 | 2 | 1.13 | 0.36 | 1.10 | 1.08 |
| PRINCEG_MD | 1 | 1 | 3 | 1.31 | 0.30 | 0.74 | 0.76 |
| ARL_VA | 0 | 0 | 1 | 0.00 | 0.36 | 0.97 | 0.87 |
| ARL_VA | 0 | 0 | 2 | 0.00 | 0.36 | 0.97 | 0.87 |


| ARL_VA | 0 | 0 | 3 | 0.00 | 0.36 | 0.97 | 0.87 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| ARL_VA | 0 | 1 | 1 | 1.22 | 0.36 | 0.97 | 0.87 |
| ARL_VA | 0 | 1 | 2 | 1.22 | 0.36 | 0.97 | 0.87 |
| ARL_VA | 0 | 1 | 3 | 1.22 | 0.36 | 0.97 | 0.87 |
| ARL_VA | 1 | 0 | 1 | 0.00 | 1.29 | 1.82 | 0.87 |
| ARL_VA | 1 | 0 | 2 | 0.00 | 0.69 | 2.31 | 0.67 |
| ARL_VA | 1 | 0 | 3 | 0.00 | 0.36 | 0.97 | 0.87 |
| ARL_VA | 1 | 1 | 1 | 1.15 | 0.46 | 1.05 | 1.18 |
| ARL_VA | 1 | 1 | 2 | 1.14 | 0.37 | 0.90 | 0.92 |
| ARL_VA | 1 | 1 | 3 | 1.44 | 0.28 | 1.30 | 0.72 |
| ALEX_VA | 0 | 0 | 1 | 0.00 | 0.36 | 1.04 | 0.81 |
| ALEX_VA | 0 | 0 | 2 | 0.00 | 0.36 | 1.04 | 0.81 |
| ALEX_VA | 0 | 0 | 3 | 0.00 | 0.36 | 1.04 | 0.81 |
| ALEX_VA | 0 | 1 | 1 | 1.24 | 0.36 | 1.04 | 0.81 |
| ALEX_VA | 0 | 1 | 2 | 1.24 | 0.36 | 1.04 | 0.81 |
| ALEX_VA | 0 | 1 | 3 | 1.24 | 0.36 | 1.04 | 0.81 |
| ALEX_VA | 1 | 0 | 1 | 0.00 | 0.57 | 1.67 | 0.81 |
| ALEX_VA | 1 | 0 | 2 | 0.00 | 0.57 | 1.62 | 0.67 |
| ALEX_VA | 1 | 0 | 3 | 0.00 | 0.36 | 1.04 | 0.81 |
| ALEX_VA | 1 | 1 | 1 | 1.13 | 0.37 | 0.97 | 1.08 |
| ALEX_VA | 1 | 1 | 2 | 1.25 | 0.35 | 1.17 | 0.82 |
| ALEX_VA | 1 | 1 | 3 | 1.43 | 0.48 | 0.88 | 0.63 |
| FALLS_FAIR_VA | 0 | 0 | 1 | 0.00 | 0.34 | 1.00 | 0.93 |
| FALLS_FAIR_VA | 0 | 0 | 2 | 0.00 | 0.34 | 1.00 | 0.93 |
| FALLS_FAIR_VA | 0 | 0 | 3 | 0.00 | 0.34 | 1.00 | 0.93 |
| FALLS_FAIR_VA | 0 | 1 | 1 | 1.21 | 0.34 | 1.00 | 0.93 |
| FALLS_FAIR_VA | 0 | 1 | 2 | 1.21 | 0.34 | 1.00 | 0.93 |
| FALLS_FAIR_VA | 0 | 1 | 3 | 1.21 | 0.34 | 1.00 | 0.93 |
| FALLS_FAR_VA | 1 | 0 | 1 | 0.00 | 1.08 | 1.84 | 0.87 |
| FALLS_FAR_VA | 1 | 0 | 2 | 0.00 | 0.80 | 1.99 | 0.61 |
| FALLS_FAR_VA | 1 | 0 | 3 | 0.00 | 0.74 | 2.09 | 0.66 |
| FALLS_FAR_VA | 1 | 1 | 1 | 1.27 | 0.22 | 1.20 | 1.10 |
| FALLS_FAR_VA | 1 | 1 | 2 | 1.18 | 0.32 | 0.89 | 0.82 |
| FALLS_FAR_VA | 1 | 1 | 3 | 1.22 | 0.40 | 1.19 | 1.26 |
| LOUDOUN_VA | 0 | 0 | 1 | 0.00 | 0.34 | 1.05 | 1.08 |
| LOUDOUN_VA | 0 | 0 | 2 | 0.00 | 0.34 | 1.05 | 1.08 |
| LOUDOUN_VA | 0 | 0 | 3 | 0.00 | 0.34 | 1.05 | 1.08 |
| LOUDOUN_VA | 0 | 1 | 1 | 1.12 | 0.34 | 1.05 | 1.08 |
| LOUDOUN_VA | 0 | 1 | 2 | 1.12 | 0.34 | 1.05 | 1.08 |
| LOUDOUN_VA | 0 | 1 | 3 | 1.12 | 0.34 | 1.05 | 1.08 |
| LOUDOUN_VA | 1 | 0 | 1 | 0.00 | 0.34 | 1.05 | 1.08 |
| LOUDOUN_VA | 1 | 0 | 2 | 0.00 | 0.57 | 2.06 | 0.57 |
| LOUDOUN_VA | 1 | 0 | 3 | 0.00 | 0.26 | 1.32 | 0.42 |
| LOUDOUN_VA | 1 | 1 | 1 | 1.11 | 0.23 | 0.45 | 1.08 |
| LOUDOUN_VA | 1 | 1 | 2 | 1.11 | 0.24 | 1.05 | 1.21 |
| LOUDOUN_VA | 1 | 1 | 3 | 1.06 | 0.58 | 1.36 | 0.91 |
| MANAS_PRINCEW | 0 | 0 | 1 | 0.00 | 0.31 | 1.01 | 1.01 |
| VA |  |  |  |  |  |  |  |
| MANAS_PRINCEW | 0 | 0 | 2 | 0.00 | 0.31 | 1.01 | 1.01 |
| VA |  |  |  |  |  |  |  |


| MANAS_PRINCEW <br> VA | 0 | 0 | 3 | 0.00 | 0.31 | 1.01 | 1.01 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| MANAS_PRINCEW <br> VA | 0 | 1 | 1 | 1.21 | 0.31 | 1.01 | 1.01 |
| MANAS_PRINCEW <br> VA | 0 | 1 | 2 | 1.21 | 0.31 | 1.01 | 1.01 |
| MANAS_PRINCEW <br> VA | 0 | 1 | 3 | 1.21 | 0.31 | 1.01 | 1.01 |
| MANAS_PRINCEW <br> VA | 1 | 0 | 1 | 0.00 | 0.31 | 1.01 | 1.01 |
| MANAS_PRINCEW <br> VA | 1 | 0 | 2 | 0.00 | 0.56 | 1.90 | 0.58 |
| MANAS_PRINCEW <br> VA | 1 | 0 | 3 | 0.00 | 1.01 | 2.31 | 0.53 |
| MANAS_PRINCEW <br> VA | 1 | 1 | 1 | 1.22 | 0.31 | 1.01 | 1.01 |
| MANAS_PRINCEW <br> VA | 1 | 1 | 2 | 1.21 | 0.33 | 0.94 | 0.85 |
| MANAS_PRINCEW <br> VA | 1 | 1 | 3 | 1.22 | 0.30 | 1.20 | 1.26 |
| SPOT_STAF_VA | 0 | 0 | 1 | 0.00 | 0.25 | 0.69 | 0.85 |
| SPOT_STAF_VA | 0 | 0 | 2 | 0.00 | 0.25 | 0.69 | 0.85 |
| SPOT_STAF_VA | 0 | 0 | 3 | 0.00 | 0.25 | 0.69 | 0.85 |
| SPOT_STAF_VA | 0 | 1 | 1 | 1.33 | 0.25 | 0.69 | 0.85 |
| SPOT_STAF_VA | 0 | 1 | 2 | 1.33 | 0.25 | 0.69 | 0.85 |
| SPOT_STAF_VA | 0 | 1 | 3 | 1.33 | 0.25 | 0.69 | 0.85 |
| SPOT_STAF_VA | 1 | 0 | 1 | 0.00 | 0.25 | 0.69 | 0.85 |
| SPOT_STAF_VA | 1 | 0 | 2 | 0.00 | 0.75 | 0.69 | 0.85 |
| SPOT_STAF_VA | 1 | 0 | 3 | 0.00 | 0.25 | 0.69 | 0.85 |
| SPOT_STAF_VA | 1 | 1 | 1 | 1.33 | 0.25 | 0.69 | 0.85 |
| SPOT_STAF_VA | 1 | 1 | 2 | 1.31 | 0.23 | 0.33 | 0.78 |
| SPOT_STAF_VA | 1 | 1 | 3 | 1.27 | 0.32 | 1.14 | 1.09 |

Table 3.1: Trip Production Rates by County

## Application of Production Trip Rates: Population Synthesis

In order to apply the trip generation model, it is necessary to have estimates of the number of individuals in each of the cross-classified categories for which the trip production rates were developed. Normally, trip production rates are applied with zonal data. However, data on individuals within households of various types is not provided on a zonal basis by the Census and surveys have samples that are too small to provide these small area estimates so an alternative method was needed. This method uses a technique known as population synthesis to create the data for all of the zones in the MWCOG region.

1. In population synthesis, disaggregate data is expanded to match known marginal totals or aggregates at the small area level. There is population synthesis tool in TransCAD used to generate a person level and household
level database for a given region. This is done with individual data records with sample weights. These are usually available from a survey. These sample records contain rich household and person information but may have little or no information household location. The Public Use Microdata Sample PUMS database from the U.S Census Bureau is often used in population synthesis and was used in the project.
2. A zonal file that contains aggregate marginal household statistics. Typical examples are the block and the block group geographic files with these marginals. These data provide geographic specificity but only for totals and subtotals.

The population synthesis procedure expands the sample files to generate the entire population or household database such that certain household and person statistics match on a zonal basis. Thus the resulting file is rich in household, individual, and geographic information. Trip rates can easily be applied to the synthesized population either on a household or persons by person basis or basis on new crosstabs at the zonal level.

For the PG County TransForM model, a special population synthesis procedure was used to match selected household and population demographics at both the block group and the block level.

The household and person demographic variables chosen for matching marginals at the block and block group level are listed below.

## Block Group:

- Household Income: Six categories of HH Income were considered:

1. Income < 20 K
2. Income between 20 K and 35 K
3. Income Between 35 K and 45 K
4. Income between 45 K and 60 K
5. Income between 60 K and 100 K
6. Income $>100 \mathrm{~K}$

- Number of Vehicles in the household: Four categories were considered. Households with $0,1,2$ and $3+$ vehicles
- Number of Workers in the household: Five categories were considered. Households with $0,1,2,3$ and 4+ workers


## Block:

- Household Size: Five categories were employed. Households with a size of 1, 2, 3, 4 and 5+
- Household Tenure: Owner/Rental
- Age of Person: Four categories were used. Age 15-34, 35-54, 55-64, 65+
- Race: White, Black, Asian, Other
- Hispanic, Non-Hispanic
- Number of children

The resulting household file and the population file contain as many records as the number of households in the region and the population of the region respectively. Each record has a corresponding block and block group ID, so that the aggregated demographics by block and block group match the marginals at the block and block group level for the above variables.

The trip production rates in Table 1 are then applied to produce productions for the HBW, HBS, HBO and NHB trip purposes by zone.

The HBW trips were further split into four categories based on income grouping. This is required for the income-stratified trip distribution model for the HBW purpose. The factors to split the HBW trips into the income categories were generated based on the earlier MWCOG TP+ model and used as inputs.

The specific software for computing trip productions was a GISDK script that performs the functions of the MWCOG FORTRAN model. This included the trip attraction models which are described below.

## Trip Attraction Rates

The trip attractions rates used in the model are those recommended by COG in the Version 2.1 \#50 Draft Calibration Report. It was found that the COG 2000 survey data was inadequate to perform regression models at the zonal level and the attraction models developed based on Area Type and various types of employment developed by COG were used. The trip rates for the basic trip purposes are listed in Table 3.2.

The HBW trip rates include both motorized as well as non-motorized attractions, whereas the trip rates for the other purposes constitute only motorized trips. The NHB trip rates represent one half of the total trip ends.

| Trip <br> Purpose | Area <br> Type | Tndependent Variables <br> Employment |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Non-Retail <br> Employment | Household <br> Population |  |  |
| HBW |  | 1.11 |  |  |  |
| HBS |  |  | 0.29 |  |  |
|  | 2 |  | 2.44 |  |  |
|  | $3-7$ |  | 3.35 |  | 0.77 |
| HBO | All (1-7) |  | 1.30 | 0.30 | 0.28 |
| NHB | 1 |  |  | 0.42 | 0.49 |

Table 3.2: Trip Attraction Rates by Trip Purpose and Area Type

## Computing Trip Attractions

The demographics of the zones are used in conjunction with the above rates to generate the trip attractions for the basic purposes. As in the case of HBW productions, the HBW trip purpose attractions are further split into the 4 income groups based on the factors developed by COG and used as inputs.

## Truck Trip Rates

Since a separate truck survey was unavailable, the truck trip rates in the model are identical to the ones developed by COG. These rates are listed in Table 3.3. They are based on fixed area types and land activity variables.

| Vehicle <br> Type | Location <br> Type | Land Use Variables |  |  |  |  |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: |
|  |  | Retail | Industrial | Other | HH |  |
| Medium Trucks <br> (Single Unit 6+ Tires) |  | 0.01 | 0.17 | 0.09 | 0.04 | 0.04 |
|  |  | 0.01 | 0.17 | 0.19 | 0.04 | 0.04 |
|  | 3-VA 10 mi-sq | 0.01 | 0.17 | 0.14 | 0.04 | 0.04 |
|  | 4-Other | 0.01 | 0.17 | 0.11 | 0.04 | 0.04 |
| Heavy Trucks <br> (All Combination <br> Vehicles) | 1-Regional Core | - | 0.04 | 0.03 | 0.03 | - |
|  | 2-DC Non-Core | - | 0.04 | 0.13 | 0.03 | - |
|  | 3-VA 10 mi-sq | - | 0.04 | 0.04 | 0.03 | - |
|  | 4-Other | - | 0.04 | 0.11 | - |  |

Table 3.3: Truck Trip Rates
The truck trips above represent internal trips. The truck internal productions are set to the truck internal attractions.

## Computing Motorized HBW Productions and Attractions

The HBW productions and attractions include motorized as well as non-motorized trips. To account for this, the non-motorized percentages have to be subtracted out. The COG 2000 Panel Survey was used to determine the walk and the bike percentages based upon area types. These percentages were similar to the ones used in the COG model and are shown in Table 3.4.

| Area Type | Non-Motorized Trip Percentage |
| :---: | :---: |
| 1 (Dense Urban Areas) | 40.21 |
| 2 | 7.52 |
| 3 | 2.61 |
| $4-7$ (Rural Areas) | 1.21 |

Table 3.4: Non-Motorized HBW Percentages

These factors were used to subtract out the non-motorized productions for the HBW internal trips.

The non-motorized trip attractions were determined using COG's approach, whereby the non-motorized attractions are set to 0.89 times the non-motorized productions.

## Computing Internal-External and External-Internal Trips

The productions and attractions for the basic trip purposed also include Internal-External (IX) trips as well as External Internal (XI) trips. In order to determine the fraction of IX and XI trips from each zone, the entire region was divided into several counties. The CTPP Part 3 journey to work estimates was observed and the percentage of work flows from a particular county to outside the region was examined.

It was found that for most of the region, the percent of IX journey to work flows was 1.2 percent. However, for counties in the outlying region (both in Virginia and Maryland), the percent of IX trips was much greater. This is because of the presence of large urban areas (such as Baltimore City) lying outside the study region that attract work trips from the outlying counties. Based on the study, the percentage of IX trips for the HBW purpose (and assumed for the other purposes) is provide in Table 3.5

| County | IX Percentage |
| :---: | :---: |
| Carroll, MD | 0.285 |
| Clarke, VA | 0.271 |
| Howard, MD | 0.193 |
| Jefferson, WVA <br> Frederick, MD <br> Loudoun, VA | 0.150 |
| Anne Arundel, MD | 0.144 |
| King George, VA | 0.019 |
| Fauquier, VA | 0.015 |
| Rest of the region | 0.012 |

## Table 3.5: Internal-External and External-Internal Trip Percentages

The above percentages are used on the production ends to determine IX productions and are used on the attraction ends to determine XI attractions.

The IX attractions and the XI productions are determined from 2000 ground counts at locations near the external stations and are provided as External Attractions and External Productions respectively.

## Trip Balancing

Trip balancing is required for the HBW, HBS, HBO and the NHB trip purposes. For the Internal-Internal trips for all these trip purposes, attractions were balanced to match
production totals. For the IX component of the above purposes, productions were balanced to external attractions whereas for the XI component, attractions were balanced to productions. Table 3.6 shows the final production and attractions

| Purpose | Internal <br> Ps and As | IX and XI <br> Ps and As | Total <br> Ps and As |
| :--- | :---: | :---: | :---: |
| HBW | $3,434,394$ | 321,331 | $3,755,725$ |
| HBS | $2,467,887$ | 124,457 | $2,592,344$ |
| HBO | $6,684,305$ | 399,859 | $7,084,164$ |
| NHB | $4,323,063$ | 201,346 | $4,524,409$ |
| MedTRK | 287,977 | - | 287,977 |
| HvyTRK | 113,362 | - | 113,362 |
| All Purposes | $\mathbf{1 7 , 3 1 0 , 9 8 8}$ | $\mathbf{1 , 0 4 6 , 9 9 3}$ | $\mathbf{1 8 , 3 5 7 , 9 8 1}$ |

Table 3.6: Final Productions and Attractions by Trip Purpose

Table 3.7 below compares the trip generation results from the PGC TransForM model with the MWCOG model for the year 2000.

| Purpose | MWCOG Trips | PGC TransForM <br> Trips |
| :--- | :---: | :---: |
| HBW | $4,150,703$ | $3,755,725$ |
| HBS | $3,123,513$ | $2,592,344$ |
| HBO | $9,532,335$ | $7,084,164$ |
| NHB | $6,978,853$ | $4,524,409$ |
| MedTRK | 304,826 | 287,977 |
| HvyTRK | 159,340 | 113,362 |
| All Purposes | $\mathbf{2 4 , 2 4 9 , 5 7 0}$ | $\mathbf{1 8 , 3 5 7 , 9 8 1}$ |

Table 3.7: Comparison of MWCOG and PGC TransForM model results
As seen in the above table, the PGC model computes fewer productions and attractions for all the trip purposes. The PGC model uses person trip rates while the MWCOG model uses household trip rates and it is likely that using household trip rates overestimates the number of trips. Further, in the MWCOG model, the HBO and NHB trips from the cross classification process were factored by 1.25 and 1.5 .

Tables 3.8 and 3.9 show the total productions and total attractions by major jurisdiction

| Jurisdiction | HBW <br> Prods | HBS <br> Prods | HBO <br> Prods | NHB <br> Prods | Truck <br> Prods | Total <br> Prods |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| DC | 274,351 | 212,266 | 574,608 | 525,853 | 54,517 | $1,641,595$ |
| Montgomery | 558,463 | 458,281 | $1,049,369$ | 660,087 | 50,432 | $2,776,632$ |
| Prince Georges | 492,291 | 320,304 | 886,148 | 575,882 | 50,118 | $2,324,743$ |
| Arlington | 136,040 | 75,215 | 216,327 | 194,942 | 12,338 | 634,863 |
| Alexandria | 92,197 | 51,077 | 137,679 | 123,867 | 10,149 | 414,969 |
| Fairfax | 638,569 | 528,235 | $1,492,926$ | 779,185 | 54,729 | $3,493,643$ |
| Loudoun | 98,284 | 64,808 | 216,912 | 136,646 | 12,997 | 529,647 |
| Prince William | 206,127 | 191,414 | 523,540 | 211,217 | 19,094 | $1,151,393$ |
| Frederick | 118,105 | 77,254 | 211,759 | 142,727 | 13,110 | 562,956 |
| Howard | 170,530 | 129,055 | 296,102 | 186,387 | 19,661 | 801,736 |
| Anne Arundel | 313,568 | 197,414 | 539,703 | 348,383 | 37,072 | $1,436,141$ |
| Charles | 70,918 | 36,353 | 139,635 | 93,779 | 7,564 | 348,249 |
| Carroll | 89,379 | 59,674 | 164,807 | 90,828 | 8,682 | 413,370 |
| Calvert | 49,457 | 12,755 | 81,839 | 48,512 | 4,001 | 196,564 |
| St. Mary's | 54,815 | 14,494 | 92,571 | 65,359 | 5,522 | 232,762 |
| King George | 10,282 | 5,147 | 12,680 | 12,029 | 774 | 40,912 |
| Fredericksburg | 11,623 | 5,377 | 12,210 | 33,117 | 2,488 | 64,816 |
| Stafford | 60,091 | 27,468 | 72,943 | 70,777 | 5,750 | 237,028 |
| Spotsylvania | 49,496 | 25,198 | 59,831 | 55,534 | 4,903 | 194,961 |
| Fauquier | 30,112 | 21,226 | 71,319 | 36,129 | 3,114 | 161,900 |
| Clarke | 6,964 | 4,722 | 15,862 | 9,034 | 939 | 37,521 |
| Jefferson | 22,834 | 16,209 | 54,439 | 27,383 | 2,503 | 123,368 |
| Total | $3,554,496$ | $2,533,946$ | $\mathbf{6 , 9 2 3 , 2 0 8}$ | $4,427,656$ | 380,459 | $\mathbf{1 7 , 8 1 9 , 7 6 6}$ |
|  |  |  |  |  |  |  |

Table 3.8: Total Productions by Trip Purpose and Jurisdiction

| Jurisdiction | HBW <br> Attrs | HBS <br> Attrs | HBO <br> Attrs | NHB <br> Attrs | Truck <br> Attrs | Total <br> Attrs |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| DC | 795,189 | 162,119 | 822,763 | 525,752 | 54,517 | $2,360,340$ |
| Montgomery | 500,914 | 356,836 | $1,023,510$ | 659,908 | 50,432 | $2,591,600$ |
| Prince Georges | 363,345 | 397,714 | 917,487 | 575,740 | 50,118 | $2,304,405$ |
| Arlington | 193,445 | 90,097 | 255,852 | 194,898 | 12,338 | 746,631 |
| Alexandria | 95,200 | 76,747 | 166,549 | 123,839 | 10,149 | 472,483 |
| Fairfax | 625,186 | 428,440 | $1,194,706$ | 778,937 | 54,729 | $3,081,998$ |
| Loudoun | 96,758 | 96,726 | 204,600 | 136,610 | 12,997 | 547,690 |
| Prince William | 118,162 | 141,750 | 359,771 | 211,140 | 19,094 | 849,917 |
| Frederick | 107,524 | 86,676 | 224,993 | 142,680 | 13,110 | 574,983 |
| Howard | 144,593 | 105,347 | 284,801 | 185,569 | 19,661 | 739,972 |
| Anne Arundel | 271,157 | 191,412 | 549,359 | 347,488 | 37,072 | $1,396,488$ |
| Charles | 53,394 | 76,435 | 141,741 | 93,746 | 7,564 | 372,879 |
| Carroll | 59,022 | 43,806 | 156,617 | 89,973 | 8,682 | 358,100 |
| Calvert | 27,469 | 32,516 | 82,500 | 48,495 | 4,001 | 194,981 |
| St. Mary's | 49,845 | 41,384 | 100,208 | 65,339 | 5,522 | 262,298 |
| King George | 11,955 | 4,417 | 19,492 | 12,023 | 774 | 48,660 |
| Fredericksburg | 23,977 | 36,826 | 31,803 | 33,113 | 2,488 | 128,208 |
| Stafford | 26,706 | 69,614 | 106,476 | 70,754 | 5,750 | 279,299 |
| Spotsylvania | 28,914 | 45,834 | 87,384 | 55,520 | 4,898 | 222,550 |
| Fauquier | 22,415 | 22,742 | 61,234 | 36,115 | 3,114 | 145,620 |
| Clarke | 6,374 | 5,254 | 13,983 | 8,972 | 939 | 35,523 |
| Jefferson | 17,091 | 16,764 | 45,654 | 27,299 | 2,503 | 109,311 |
| Total | $\mathbf{3 , 6 3 8 , 6 3 6}$ | $\mathbf{2 , 5 2 9 , 4 5 3}$ | $\mathbf{6 , 8 5 1 , 4 8 4}$ | $\mathbf{4 , 4 2 3 , 9 0 9}$ | 380,454 | $\mathbf{1 7 , 8 2 3 , 9 3 7}$ |
|  |  |  |  |  |  |  |

Table 3.9: Total Attractions by Trip Purpose and jurisdiction

Figure 3.1 shows a pie chart for the production and attractions for the entire region, while Figure 3.2 depicts the productions and attractions for PG County.


Figure 3.1: Productions and Attractions for the entire region


Figure 3.2: Total Production and Attractions for Prince George's County

Figures 3.3 and 3.4 show the productions and attractions of work trips (HBW trips) for the entire region and for PGCounty respectively.


Figure 3.3: HBW Productions and Attractions for the entire region


Figure 3.4: HBW Productions and Attractions for Prince George's County

## Chapter 4 Trip Distribution

This chapter describes the development of the trip distribution models that were implemented in the TransForM model. The chapter describes the MWCOG Trip Distribution model and then focuses on the changes implemented for the TransForM model.

## The MWCOG Trip Distribution Model

The MWCOG Trip Distribution model employs a TP+ gravity procedure and a composite (highway and transit) travel time impedance measure.

The internal productions and attractions are obtained for each of the four income categories from the trip generation procedure for inputs into the trip distribution process for the HBW, HBS and the HBO purposes. The internal productions and attractions are not split into the income categories for the NHB, Medium Truck and the Heavy Truck purposes. In addition, productions and attractions for the IX and XI trips are computed from the trip generation process.

The friction factors curves for the HBW, HBO, HBS and NHB trips were developed by COG using the COG 1994 Panel Survey. Four sets of friction factors corresponding to trips made by households in each income classification were developed for the HBW, HBS, and HBO purposes. The truck survey was used to develop friction factor curves for the truck purposes. For the IX and XI trips, friction factor curves were developed for trips that travel to external stations using interstates and for trips that travel to external stations using arterials.

The friction factors are smoothed using a gamma function. There are further adjustments that were used iteratively to approximate the trip length distributions observed in the 1994 survey by purpose.

The highway skims are combined with the transit skims to provide a composite skim for use for the HBW, HBS, HBO and NHB purposes. Further details on this procedure are provided later in this Chapter.

In the MWCOG model, the trip distribution method employs a set of K-Factors to adjust jurisdiction to jurisdiction flows. The initial set of K-Factors was developed in 1994 and was subsequently modified in the MWCOG model as described in the Version 2.1D Draft \#50 calibration report dated September 17, 2004.

The TP+ procedure balances rows (productions) and then factors the calculated attractions so that they are normalized to match the input attractions. However, in this process, the external attractions are held fixed. Therefore, this procedure applies the gravity equation (employing the friction factors) only to the productions and not to the attractions.

Early in the project, we performed several experiments to study the behavior of the MWCOG TP+ gravity model and the TransFORM model. The TP+ method is similar to the method used in UTPS and TRANPLAN and approximates a doubly-constrained model by balancing origins and using a normalization loop on attractions. While TransCAD supports this type of gravity model, the doubly-constrained model in TransCAD was thought to be more efficient and to give more accurate answers.

The MWCOG trip distribution model is run with a maximum of 7 iterations and uses a default convergence criterion of a root mean square error (RSME) of 10 percent. We examined the output from the distribution models run in the second pass of the model (after the first pump-prime feedback loop) and discovered that most of the models were not converged. In fact, the RMSEs were rather high for most of the trip purposes, suggesting that quite a bit more computing would be indicated. We thus resolved to use the TransCAD doubly constrained gravity model and to use a much higher convergence value in the TransForM model.

## Highway and Transit Skimming

The first step before the trip distribution analysis was the generation of highway and transit skim matrices based on the updated network and route system. The peak and offpeak highway skim matrices were generated from the latest network using the peak and the off-peak congested travel times computed from the calibration runs. The skims were generated for SOV, HOV2 and HOV3+ modes and were performed for the peak and the off peak period to generate six sets of skims. The link prohibitions for the SOV vehicles and HOV2 vehicles were taken into account in building the appropriate network file to be used for the skimming process.

The congested times that are input to the skimming procedure are the result of several full model runs using the feedback procedure with the Method of Successive Averages (MSA). During each loop of the feedback process, the congested times are fed into the trip distribution procedure. After the traffic assignment stage, the output flows are averaged and updated link travel times are computed. The resulting times are fed back into the skimming procedure and the above process is repeated until some degree of convergence is achieved which implies that the travel times input to the skimming procedure are similar to the ones that are produced by the traffic assignment step. Further details of this process are provided in Chapter 7, Traffic Assignment.

The peak period skims are the average of the AM and PM skims, which is believed to be more accurate than just using the AM values.

The intra-zonal travel times are computed by using one-half of the average distance to the two neighboring zones. The intra-zonal travel times are computed after the highway skimming process during each calibration loop.

The terminal times are then added after computing the intra-zonal times. The terminal time for each zone is specified in the zone database and is based on the employment density as follows:

| Employment Density | Terminal Time (min) |
| :---: | :---: |
| $<4,600$ | 1.0 |
| $>=4,600$ and $<6,600$ | 2.0 |
| $>=6,600$ and $<11,500$ | 3.0 |
| $>-11,500$ and $<33,000$ | 4.0 |
| $>=33,000$ | 5.0 |

Table 4.1 Zone Terminal Times based on Employment Density
The terminal times are based on the same classifications as in the MWCOG model, however the values of the terminal times used are smaller. The MWCOG model employed as high as 8 minutes of terminal times for the zones with the highest employment density.

For the transit skims, the TransCAD Pathfinder method developed by Caliper was employed on the latest route system to generate the peak and the off-peak transit skims. Pathfinder takes account of the reduced first wait for overlapping services and calculates the travel time components and number of transfers for the best paths between origins and destinations. The Pathfinder method also takes the fare into account in finding the best transit path which is another difference from the MWCOG model. A default value of time of $\$ 12$ per hour is used although this could be fine-tuned further by income category if desired.

To obtain composite times, the skim matrices were combined using the composite impedance equation as per the MWCOG model for the HBW trip purpose
$1.0 / \mathrm{CT}_{\mathrm{i}}=1.0 / \mathrm{HT}+\mathrm{P}_{\mathrm{i}} / \mathrm{TT}$, where
$\mathrm{CT}_{\mathrm{i}}=$ composite time for income group I
HT = highway time
TT = transit time
$\mathrm{P}_{\mathrm{i}}=$ Regional transit share of income group
The percentage of transit trips for household under each income type (for the HBW purpose) was obtained from the COG Panel survey and the numbers were similar to the percentages used in the MWCOG model. The transit percentage by income group for the HBW purpose is shown in the table below:

| Income Category | Percentage Transit |
| :---: | :---: |
| 1 | 25.7 |
| 2 | 14.8 |
| 3 | 13.7 |
| 4 | 14.0 |

Table 4.2 Transit Percentages by Income Group

However, unlike MWCOG model, the income stratification based trip distribution model was applied only for the HBW purpose. This is in keeping with the general notion that income does not greatly affect the distribution of non-work purposes. It was also warranted by the relatively small sample size of the panel survey. For the other purposes the off peak skims were directly used in evaluating the distribution model equations.

## Friction Factor Curves and Calibration

The MWCOG model used gamma curves for the friction factors, and the same functional form was used in the TransForM model. The friction factors were adjusted for the Year 2000 using the panel survey.

In order to calibrate the friction factors, the origin and the destination TAZ of each trip in the survey database were identified using TransCAD's geocoding tools. Thus, the travel time for each trip could be computed from the skim matrices. The average travel times were then extracted from the survey for each of the purposes. For the HBW purpose, additional average travel times were obtained for each of the four household income classifications. The average times for the internal trips for each of the purposes from the survey are listed in Table 4.3.

The MWCOG friction factor curves for each of the trip purposes were then adjusted to yield the above estimates of average trip lengths. In particular it was observed that the trip lengths from the MWCOG model were longer than desired and hence the curves were shifted to the left to yield lower trip lengths.

| Trip Purpose | Avg Trip Length (min) |
| :--- | :---: |
| HBW - Income 1 | 19.0 |
| HBW - Income 2 | 21.7 |
| HBW - Income 3 | 25.5 |
| HBW - Income 4 | 26.0 |
| HBW - All Incomes | 24.3 |
| HBS | 11.9 |
| HBO | 15.5 |
| NHB | 13.8 |

Table 4.3 Observed Trip Lengths from the 2000 COG Panel Survey
It must be emphasized that in order to calibrate the above friction factor curves, several model runs had to be performed since the friction factors depend on the peak and offpeak congested travel times, which in turn depend on the traffic assignment results. These congested travel times were thus constantly updated during the calibration process and hence the friction factors were also adjusted on a regular basis. The final congested times after several model runs were used to calibrate the final friction factor curves. The friction factor curves for the four purposes are shown in the figures below.


Figure 4.1 HBW Friction Factors


Figure 4.2 HBS Friction Factors

HBO Friction Factors


Figure 4.3 HBO Friction Factors


Figure 4.4 NHB Friction Factors

## Trip Distribution for the HBW, HBS, HBO and NHB purposes

The doubly constrained gravity procedure was applied to all the Production and Attraction vectors with the appropriate friction factor curves. The composite impedance matrices were used for the HBW purpose, whereas the off peak impedance matrices were used for the other purposes. The procedure was run until convergence with a tolerance of 0.001 .

The TransForM Trip Distribution model does not employ K Factors for the 4 basic trip purposes. This is an advantage of this model since the extensive usage of K Factors is generally not recommended.

## Truck Trip Distribution

Since no information was available regarding the truck trips, the friction factors and the K Factors for the trucks as in the MWCOG model were employed. The only difference is that the truck trip distribution uses the latest impedance matrix.

## IX and XI Trip Distribution

The IX and XI trips for all the purposes are split into two main categories: Trips through interstates known as EI trips and trips through arterials known as EA trips. The friction factor curves are different for the above categories and in general the EA friction factor curve falls more steeply than the EI friction factor curve (as seen in the figures above). Further, each external station in the model is categorized to one of these two types. External stations that are connected to the network by major arterials fall under the EA category, while external stations connected to the network by interstates fall under the EI category. The gravity model is applied twice to the IXXI trips, once with the EI friction factor curve and once with the EA friction factor curve to produce the EA and the EI gravity matrix for each purpose. The impedance matrices applied during the process are modified so that Internal-Internal (II) and External-External (XX) trips have a high impedance to prevent any allocation of trips to these regions. Then depending on the category of each external station, either the EA or the EI matrix for that particular station (row and column) is zeroed out.

Finally, after the procedure is completed, the resulting matrix is not subject to the bucket rounding method as in the MWCOG model.

## Trip Distribution Results

## Survey versus Model Trip Percentages

Figures 4.5 to 4.8 show the percentage of trips in each time interval by purpose obtained from the survey and those obtained from the model after the trip distribution procedure (using the output matrix and the skims). Each point in the figures below represents the
percent of trips that have the corresponding travel time on the X -axis (within a 1 minute interval).

HBS Trip Percentages: Survey versus Model Trips


Figure 4.5 HBW Trips: Survey versus Model Trip Frequencies
HBS Trip Percentages - Survey versus Model Trips


Figure 4.6 HBS Trips: Survey versus Model Trip Frequencies

HBO Trip Percentages - Survey versus Model Trips


Figure 4.7 HBO Trips: Survey versus Model Trip Frequencies

NHB Trip Percentages: Survey versus Model Trips


Figure 4.8 NHB Trips: Survey versus Model Trip Frequencies

Table 4.4 shows the number of trips for each purpose along with the number of intrazonal trips, the intra-zonal trip percentage, the average trip length in miles and the average trip duration in minutes.

| Trip Purpose | Number <br> Trips | Number <br> IZ Trips | Pct <br> IZ Trips | Avg Trip <br> Duration | Avg Trip <br> Dur. (Survey) |
| :--- | :---: | :---: | :---: | :---: | :---: |
| HBW - Inc 1 | 548,113 | 34,103 | 6.2 | 18.9 | 19.0 |
| HBW - Inc 2 | 861,414 | 54,349 | 6.3 | 22.0 | 21.7 |
| HBW - Inc 3 | 724,651 | 38,024 | 5.2 | 25.0 | 25.5 |
| HBW - Inc 4 | $1,300,217$ | 48,873 | 3.8 | 26.9 | 26.0 |
| HBW - Total (with <br> EI and EA trips) | $\mathbf{3 , 7 5 5 , 7 2 5}$ | $\mathbf{1 7 5 , 3 4 9}$ | $\mathbf{4 . 7}$ | $\mathbf{2 6 . 0}$ | $\mathbf{2 4 . 3}$ |
| HBS - Total | $\mathbf{2 , 5 9 2 , 3 4 5}$ | $\mathbf{3 4 6 , 9 4 6}$ | $\mathbf{1 3 . 4}$ | $\mathbf{1 4 . 1}$ | $\mathbf{1 1 . 9}$ |
| HBO - Total | $\mathbf{7 , 0 8 4 , 1 6 4}$ | $\mathbf{1 , 3 0 9 , 2 0 2}$ | $\mathbf{1 9 . 6}$ | $\mathbf{1 4 . 7}$ | $\mathbf{1 5 . 5}$ |
| NHB - Total | $\mathbf{4 , 5 2 5 , 4 0 9}$ | $\mathbf{7 0 4 , 7 6 2}$ | $\mathbf{1 5 . 6}$ | $\mathbf{1 5 . 0}$ | $\mathbf{1 3 . 8}$ |
| MedTrk - Total | $\mathbf{2 8 7 , 9 7 7}$ | $\mathbf{1 3 , 2 8 1}$ | $\mathbf{4 . 6}$ | $\mathbf{2 7 . 3}$ | - |
| HvyTrk - Total | $\mathbf{1 1 3 , 3 6 5}$ | $\mathbf{3 , 6 0 0}$ | $\mathbf{3 . 2}$ | $\mathbf{3 0 . 4}$ | - |

Table 4.4 Trip Distribution Results
The estimated trip distribution between and within jurisdictions for the HBW purpose is shown in Figure 4.9. Tables 4.3 to 4.9 indicate the estimated number of trips between various jurisdictions for each of the purposes.


Figure 4.9 HBW Trip Patterns

|  | DC | MTG | PGC | ARL | ALX | FFAX |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| DC | 165,984 | 33,421 | 29,089 | 18,904 | 5,049 | 15,632 |
| MTG | 142,854 | 279,207 | 46,160 | 18,599 | 4,286 | 32,156 |
| PGC | 184,100 | 46,899 | 164,541 | 22,60 | 7,918 | 18,171 |
| ARL | 4,432 | 9,352 | 4,216 | 32,703 | 8,448 | 34,030 |
| ALX | 24,728 | 3,720 | 3,730 | 16,298 | 16,062 | 25,766 |
| FFAX | 96,407 | 31,554 | 14,173 | 54,818 | 36,981 | 351,876 |
| LDN | 5,298 | 3,578 | 454 | 2,493 | 758 | 34,948 |
| PW/MN | 15,535 | 3,555 | 2,057 | 9,195 | 7,636 | 69,274 |
| FRD | 6,146 | 25,557 | 1,401 | 893 | 168 | 2,943 |
| HWD | 20,547 | 2,632 | 19,955 | 2,106 | 482 | 2,062 |
| ANNE | 37,925 | 12,426 | 44,933 | 3,963 | 1,072 | 2,248 |
| CHL | 17,409 | 1,339 | 11,712 | 2,421 | 1,292 | 1,931 |
| CAR | 2,688 | 11,785 | 1,913 | 301 | 57 | 521 |
| CAL | 7,247 | 848 | 7,817 | 918 | 316 | 482 |
| STM | 2,537 | 151 | 2,887 | 303 | 152 | 183 |
| KGRG | 599 | 34 | 363 | 83 | 55 | 113 |
| FBURG | 99 | 14 | 15 | 63 | 61 | 321 |
| STF | 3,090 | 460 | 408 | 1,808 | 1,667 | 9,051 |
| SPT | 1,112 | 183 | 202 | 680 | 652 | 3,026 |
| FQR | 732 | 350 | 70 | 495 | 252 | 6,983 |
| CLK | 42 | 60 | 3 | 23 | 10 | 617 |
| JF | 187 | 832 | 33 | 56 | 16 | 991 |
| EXT | 15,135 | 9,791 | 6,982 | 3,762 | 1,868 | 12,323 |
|  |  |  |  |  |  |  |


| LDN | PW/MN |
| :---: | :---: |
| 267 | 50 |
| 1,320 | 99 |
| 269 | 86 |
| 1,019 | 163 |
| 308 | 389 |
| 31,940 | 11,674 |
| 43,232 | 1,471 |
| 7,368 | 80,051 |
| 2,935 | 60 |
| 73 | 5 |
| 29 | 7 |
| 16 | 9 |
| 162 | 3 |
| 5 | 2 |
| 3 | 1 |
| 1 | 139 |
| 5 | 693 |
| 189 | 11,823 |
| 57 | 3,864 |
| 1,734 | 5,197 |
| 1,252 | 179 |
| 2,670 | 49 |
| 2,028 | 2,165 |

FRD
47
11,642
91
17
6
270
3,120
88
70,726
2,710
122
1
13,155
2
1
0
0
3
5
36
1117
3,221
2,152
HWD
808
10,469
8,380
56
25
303
61
26
2,980
51,293
16,587
91
7,363
92
14
2
0
3
21
2
3
54
44,799
ANNE
1,241
5,76
22,192
71
54
422
18
43
472
16,134
158,994
655
1,186
3,152
219
14
0
7
76
2
0
6
60,733
CHL
622
134
9,904
42
100
572
6
74
2
42
732
30,002
3
1,907
5,960
1,611
42
296
387
3
0
0
1,004
CAR
2
379
9
0
0
0
2
9
0
1,738
986
28
0
28,253
0
0
0
0
0
6
0
0
11
28,585
CAL
51
29
1,552
3
5
30
0
3
1
11
2,411
1,148
1
19,200
2,430
35
1
6
41
0
0
0
524
STM
5
2
416
1
1
7
7
1
0
1
35
1,934
0
6,951
39,310
151
4
25
152
0
0
0
924
KGRG
0
0
7
7
0
0
1
0
40
0
0
1
217
0
6
93
6,147
792
2,285
2,016
18
0
0
333
FB






QRR
3
6
6
3
7
3
3
697
552
4,095
14
0
0
0
0
1
0
0
22
152
152
2,425
557
1,820
445
62
562
CLK
0
4
0
0
0
0
25
442
31
70
1
0
0
3
0
0
0
0
3
2
92
1,957
919
2,932

EXT
3,028
5,776
4,900
1,455
978
9,727
1,114
1,999
1,256
27,605
31,875
704
23,008
499
547
158
114
573
422
376
1,690
2,285
0

| LDN | PW/MN | FRD | HWD | ANNE | CHL | CAR | CAL | STM | KGRG | FBURG |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0 | 0 | 1 | 15 | 5,807 | 0 | 152 | 1,759 | 1 | 248 |
| 169 | 0 | 4,879 | 621 | 216 | 96 | 57 | 603 | 6,797 | 4 | 1,536 |
| 0 | 0 | 0 | 507 | 4,585 | 38,615 | 0 | 8,580 | 2,854 | 0 | 20 |
| 0 | 3 | 0 | 0 | 0 | 2 | 0 | 4 | 84 | 0 | 49 |
| 0 | 18 | 0 | 0 | 0 | 76 | 0 | 2 | 91 | 0 | 41 |
| 31,383 | 19,273 | 0 | 0 | 0 | 414 | 0 | 40 | 1,386 | 2 | 6,796 |
| 62,203 | 16 | 130 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 62 |
| 910 | 120,477 | 0 | 0 | 0 | 0 | 0 | 1 | 29 | 1 | 10,178 |
| 624 | 0 | 75,138 | 2 | 0 | 0 | 47 | 3 | 21 | 0 | 27 |
| 0 | 0 | 1,756 | 88,106 | 4,519 | 19 | 454 | 142 | 1,875 | 1 | 106 |
| 0 | 0 | 0 | 1,167 | 165,196 | 16 | 0 | 8,841 | 1,827 | 0 | 3 |
| 0 | 0 | 0 | 0 | 0 | 30,761 | 0 | 1,013 | 4,246 | 8 | 0 |
| 0 | 0 | 4,714 | 941 | 0 | 129 | 34,283 | 863 | 5,261 | 2 | 772 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12,097 | 555 | 0 | 0 |
| 0 | 0 | 0 | 0 | 0 | 3 | 0 | 9 | 14,365 | 0 | 0 |
| 0 | 0 | 0 | 0 | 0 | 16 | 0 | 0 | 0 | 4,288 | 353 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4,637 |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 10,483 |
| 108 | 289 | 1 | 15 | 47 | 131 | 2 | 78 | 103 | 14 | 694 |
| 143 | 25 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 458 |
| 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 |
| 11 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 642 | 918 | 719 | 14,472 | 17,591 | 488 | 9,077 | 188 | 234 | 45 | 193 |

Table 4.6 HBS Jurisdiction to Jurisdictions Trips

|  | DC | mtg | PGC | ARL | alx | frax | LDN | PW/Mn | FRD | HwD | anne | CHL | CAR | CAL | stm | KGRG | fburg | STF | SPT | FQR | cLK | JF | Ext |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| DC | 384,549 | 54,132 | 103,252 | 10,289 | 2,828 | 4,707 | 33 | 50 | 143 | 1,842 | 4,134 | 1,462 | 25 | 356 | 90 | 37 | 19 | 32 | 346 | 2 | 0 | 1 | 6,036 |
| mtG | 93,498 | 798,466 | 80,769 | 5,030 | 1,037 | 8,297 | 349 | 35 | 14,655 | 24,187 | 7,457 | 206 | 2,548 | 142 | 167 | 112 | 62 | 104 | 531 | 8 | 1 | 13 | 11,067 |
| PGC | 135,240 | 53,371 | 598,889 | 4,155 | 2,769 | 3,924 | 17 | 53 | 126 | 13,943 | 41,437 | 16,800 | 66 | 3,672 | 1,048 | 267 | 38 | 65 | 484 | 4 | 0 | 1 | 9,305 |
| ARL | 51,155 | 9,689 | 9,554 | 90,418 | 18,744 | 32,711 | 150 | 233 | 60 | 173 | 437 | 273 | 11 | 47 | 40 | 25 | 14 | 64 | 203 | 5 | 0 | 0 | 2,267 |
| ALX | 21,651 | 2,796 | 7,950 | 25,796 | 51,120 | 25,216 | 50 | 330 | 14 | 64 | 284 | 558 | 7 | 58 | 35 | 18 | 16 | 101 | 148 | 2 | 0 | 0 | 1,426 |
| frax | 109,868 | 59,360 | 48,253 | 108,089 | 80,700 | 990,466 | 39,184 | 23,508 | 941 | 954 | 1,874 | 3,334 | 193 | 472 | 766 | 491 | 673 | 3,485 | 2,453 | 1,690 | 31 | 37 | 15,889 |
| LDN | 2,034 | 4,582 | 585 | 1,538 | 429 | 36,089 | 152,955 | 1,714 | 9,410 | 148 | 210 | 95 | 142 | 92 | 210 | 208 | 115 | 196 | 577 | 837 | 935 | 1,398 | 2,516 |
| PW/MN | 10,350 | 3,174 | 4,621 | 8,031 | 7,314 | 80,780 | 5,709 | 326,949 | 255 | 164 | 449 | 395 | 158 | 203 | 629 | 778 | 7,536 | 38,342 | 11,678 | 10,357 | 59 | 18 | 5,454 |
| FRD | 211 | 10,317 | 160 | 28 | 6 | 94 | 1,023 | 6 | 184,157 | 3,262 | 118 | 26 | 9,262 | 26 | 29 | 34 | 29 | 52 | 113 | 4 | 13 | 386 | 2,240 |
| HWD | 1,922 | 14,531 | 14,149 | 55 | 13 | 71 | 5 | 2 | 2,758 | 186,571 | 17,754 | 22 | 3,791 | 23 | 48 | 32 | 18 | 30 | 275 | 2 | 0 | 1 | 50,82 |
| anne | 3,305 | 2,909 | 27,858 | 102 | 50 | 81 | 3 | 4 | 38 | 13,280 | 426,203 | 404 | 52 | 4,312 | 124 | 58 | 32 | 54 | 510 | 3 | 0 | 1 | 60,130 |
| CHL | 1,759 | 136 | 10,924 | 97 | 141 | 176 | 1 | 3 | 10 | 23 | 426 | 113,046 | 9 | 1,841 | 5,454 | 3,797 | 39 | 69 | 188 | 2 | 0 | 0 | 1,456 |
| car | 37 | 1,384 | 95 | 5 | 2 | 10 | 4 | 2 | 5,227 | 3,201 | 105 | 11 | 110,102 | 10 | 10 | 10 | 3 | 12 | 88 | 2 | 0 | 1 | 48,084 |
| cal | 361 | 76 | 2,585 | 14 | 13 | 17 | 1 | 1 | 6 | 18 | 3,269 | 792 | 5 | 68,012 | 5,645 | 30 | 8 | 14 | 89 | 1 | 0 | 0 | 857 |
| STM | 29 | 11 | 288 | 2 | 2 | 5 | 1 | 1 | 4 | 5 | 22 | 2,933 | 2 | 2,442 | 84,863 | 836 | 12 | 20 | 111 | 1 | 0 | 0 | 962 |
| KGRG | 1 | 1 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 112 | 0 | 1 | 31 | 10,910 | 308 | 590 | 502 | 0 | 0 | 0 | 211 |
| FBURG | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 77 | 8,154 | 1,139 | 2,709 | 1 | 0 | 0 | 128 |
| STF | 7 | 4 | 5 | 5 | 6 | 34 | 0 | 257 | 2 | 2 | 5 | 4 | 2 | 2 | 8 | 1,005 | 8,615 | 50,834 | 11,249 | 135 | 0 | 0 | 746 |
| SPT | 300 | 164 | 251 | 130 | 105 | 566 | 32 | 224 | 4 | 107 | 263 | 96 | 60 | 46 | 82 | 201 | 3,783 | 1,818 | 51,023 | 15 | 4 | 4 | 604 |
| FQR | 176 | 170 | 128 | 158 | 51 | 2,516 | 1,086 | 3,761 | 72 | 59 | 152 | 70 | 55 | 71 | 144 | 245 | 2,035 | 8,600 | 3,202 | 47,265 | 278 | 34 | 1,034 |
| CLK | 27 | 35 | 33 | 6 | 3 | 59 | 823 | 42 | 162 | 15 | 40 | 17 | 15 | 10 | 6 | 41 | 22 | 38 | 111 | 291 | 9,092 | 905 | 4,707 |
| JF | 64 | 296 | 77 | 14 | 7 | 65 | 1,624 | 10 | 5,260 | 63 | 96 | 33 | 67 | 18 | 6 | 56 | 53 | 89 | 256 | 26 | 1,019 | 39,520 | 6,738 |
| Ext | 6,094 | 7,590 | 6,821 | 1,883 | 1,210 | 8,890 | 1,695 | 2,560 | 1,647 | 35,124 | 44,631 | 1,029 | 31,376 | 607 | 717 | 219 | 218 | 733 | 583 | 599 | 2,884 | 3,843 | 0 |


|  |
| :---: |
| ¢ ¢ ¢ |
| 今 |
|  |
|  |
| Q |
|  |
| 2 へ. |
|  <br>  |
|  |
|  |
|  |
|  <br>  |
|  |
|  |

STM
11
0
294
0
1
1
0
0
0
0
5
2,136
0
2,422
59,899
97
0
0
39
0
0
0
493

KGRG
0
0
11
0
0
0
0
5
0
0
0
564
0
2
113
10,197
206
500
271
2
0
0
160
FBURG
0
0
0
0
0
2
0
128
0
0
0
2
0
0
0
241
20,800
5,701
6,103
59
0
0
97
STF
0
0
0
8
23
274
0
3,202
0
0
0
4
0
0
0
546
5,784
54,952
4,603
858
0
0
540

FQR
0
0
0
4
1
1,059
599
3,067
1
0
0
0
0
0
0
2
55
843
56
29,920
128
13
371
CLK
0
0
0
0
0
0
17
461
17
29
0
0
0
0
0
0
0
0
0
1
93
6,033
713
1,706


Table 4.8 NHB Jurisdiction to Jurisdictions Trips

|  | DC | mta | PGC | ARL | ALX | frax | LDN | PW/MN | FRD | HwD | ANNE | CHL | CAR | CAL | STM | Kgrg | fburg | STF | SPT | FQR | cLK | JF | Ext |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| DC | 22,167 | 4,577 | 4,938 | 1,583 | 876 | 2,675 | 333 | 524 | 175 | 611 | 436 | 326 | 154 | 202 | 132 | 25 | 29 | 112 | 48 | 72 | 12 | 39 | 169 |
| mtG | 5,482 | 19,462 | 4,023 | 929 | 513 | 2,916 | 605 | 493 | 252 | 1,540 | 1,042 | 174 | 675 | 155 | 69 | 12 | 19 | 89 | 25 | 98 | 31 | 149 | 325 |
| PGC | 5,899 | 4,057 | 18,520 | 766 | 611 | 1,906 | 236 | 391 | 158 | 1,205 | 1,862 | 568 | 187 | 403 | 227 | 37 | 22 | 88 | 34 | 53 | 8 | 33 | 234 |
| ARL | 986 | 1,139 | 988 | 2,722 | 839 | 2,202 | 218 | 365 | 59 | 255 | 194 | 91 | 48 | 54 | 37 | 8 | 17 | 69 | 31 | 45 | 8 | 17 | 58 |
| ALX | 544 | 623 | 762 | 838 | 2,332 | 1,866 | 134 | 328 | 34 | 155 | 136 | 101 | 27 | 49 | 39 | 8 | 15 | 65 | 27 | 31 | 5 | 10 | 39 |
| ffax | 1,556 | 3,325 | 2,314 | 2,163 | 1,873 | 23,827 | 2,183 | 2,569 | 235 | 717 | 466 | 299 | 157 | 153 | 113 | 27 | 82 | 344 | 143 | 384 | 67 | 123 | 229 |
| LDN | 189 | 672 | 276 | 211 | 132 | 2,095 | 3,953 | 560 | 216 | 170 | 74 | 31 | 83 | 14 | 8 | 2 | 12 | 52 | 20 | 173 | 93 | 190 | 80 |
| PW/MN | 304 | 588 | 480 | 359 | 329 | 2,576 | 510 | 7,048 | 58 | 148 | 107 | 72 | 24 | 37 | 23 | 24 | 104 | 481 | 169 | 390 | 34 | 41 | 91 |
| FRD | 212 | 173 | 154 | 53 | 31 | 222 | 208 | 59 | 7,306 | 208 | 77 | 8 | 402 | 5 | 3 | 1 | 1 | 3 | 2 | 22 | 25 | 144 | 150 |
| HWD | 729 | 1,505 | 1,196 | 258 | 159 | 765 | 180 | 158 | 182 | 6,767 | 898 | 45 | 352 | 45 | 18 | 3 | 1 | 6 | 2 | 9 | 6 | 32 | 185 |
| ANNE | 518 | 1,068 | 1,885 | 151 | 110 | 390 | 64 | 87 | 74 | 972 | 19,367 | 122 | 131 | 218 | 77 | 10 | 2 | 12 | 4 | 7 | 1 | 11 | 334 |
| CHL | 396 | 181 | 583 | 74 | 83 | 246 | 27 | 59 | 9 | 47 | 126 | 3,469 | 7 | 139 | 209 | 48 | 18 | 34 | 30 | 7 | 0 | 1 | 33 |
| CAR | 187 | 674 | 198 | 40 | 23 | 139 | 74 | 19 | 382 | 371 | 131 | 7 | 3,372 | 6 | 5 | 1 | 1 | 4 | 3 | 5 | 9 | 49 | 344 |
| cal | 243 | 161 | 412 | 43 | 40 | 127 | 12 | 29 | 7 | 46 | 219 | 137 | 6 | 1,027 | 456 | 16 | 7 | 12 | 11 | 2 | 0 | 1 | 42 |
| STM | 161 | 75 | 232 | 30 | 32 | 92 | 7 | 18 | 3 | 19 | 78 | 207 | 5 | 458 | 2,552 | 44 | 18 | 30 | 29 | 3 | 1 | 2 | 57 |
| kgrg | 29 | 13 | 38 | 7 | 7 | 24 | 1 | 24 | 0 | 3 | 10 | 47 | 1 | 17 | 44 | 128 | 46 | 81 | 71 | 10 | 0 | 0 | 18 |
| fburg | 37 | 24 | 25 | 15 | 14 | 76 | 11 | 100 | 1 | 1 | 3 | 17 | 1 | 7 | 18 | 44 | 485 | 471 | 543 | 40 | 2 | 1 | 33 |
| STF | 142 | 101 | 99 | 61 | 60 | 317 | 44 | 453 | 4 | 7 | 16 | 34 | 3 | 13 | 30 | 81 | 467 | 1,678 | 625 | 153 | 6 | 4 | 78 |
| SPT | 66 | 36 | 42 | 27 | 26 | 134 | 17 | 164 | 2 | 2 | 4 | 30 | 3 | 11 | 30 | 71 | 545 | 610 | 1,582 | 66 | 3 | 1 | 118 |
| FQR | 102 | 107 | 63 | 46 | 32 | 368 | 147 | 381 | 19 | 12 | 10 | 9 | 4 | 3 | 3 | 10 | 41 | 153 | 68 | 640 | 31 | 29 | 56 |
| cLK | 17 | 32 | 9 | 7 | 5 | 62 | 86 | 37 | 23 | 7 | 1 | 0 | 10 | 0 | 1 | 0 | 2 | 7 | 3 | 34 | 169 | 106 | 37 |
| JF | 47 | 154 | 31 | 15 | 9 | 111 | 180 | 44 | 140 | 37 | 12 | 1 | 51 | 1 | 2 | 0 | 1 | 5 | 1 | 33 | 108 | 803 | 52 |
| Ext | 199 | 327 | 239 | 51 | 35 | 211 | 72 | 89 | 135 | 197 | 333 | 34 | 339 | 42 | 57 | 18 | 37 | 77 | 124 | 57 | 37 | 52 | 0 |

Table 4.9 Medium Truck Jurisdiction to Jurisdictions Trips




CLK
6
6
12
4
2
2
2
12
13
7
4
3
1
0
2
0
0
0
0
2
1
3
21
12
109

EXT
1,691
2,559
1,976
289
296
1,636
640
874
851
1,450
2,391
286
1,201
182
241
47
161
354
452
212
108
226
Table 4.10 Heavy Truck Jurisdiction to Jurisdictions Trips

## Chapter 5 Mode Choice

This chapter discusses the mode choice component of the TransForM model and includes a discussion of both model calibration and validation. The mode choice model is based upon the MWCOG mode choice model which is briefly described below. In the TransForM model, we use the same mode choice equations as in Version 2.1 D of the MWCOG model, but with different constants. In some exploratory estimation, we found that similar model coefficients would be obtained with the panel survey data. Unlike the MWCOG model, the models applied are pure logit models without adjustments except for the constants. Also, the mode choice models are run in each loop of the overall model run. This also produces greater consistency in the model.

## The MWCOG Mode Choice Model

When began the project, the mode choice component in the MWCOG model was not executed during every loop of the model although this has subsequently been changed.

Currently, the mode choice is performed by a FORTRAN program (COGMC.EXE) that is run once for each trip purpose (HBW, HBS, HBO and NHB). There are 3 main steps to the mode choice procedure: (1) Logit models to obtain probabilities, (2) Adjustment of logit probabilities to match district-to-district shares, and (3) application of resulting probabilities to input trips to obtain trips by each mode.

The logit model has 3 alternatives: transit, drive alone and carpool, and is applied to 21 market segments defined by:

- Transit Access
- Short walk at origin - Short walk at destination
- Short walk - Long walk
- Long walk - Short walk
- Long walk - Long walk
- Drive - Short walk
- Drive - Long walk
- No transit access
- Auto Ownership
- 0-auto household
- 1-auto household
- 2+-auto household

The resulting mode choice probabilities are then adjusted to match district totals.
After the mode choice model is run, there is an auto occupancy routine that separates SOV auto drivers into the number of 1-occupancy auto, 2-occupancy auto, and 3 or moreoccupancy auto trips for each origin-destination (OD) pair. This is performed via linear equations that are a function of auto occupancy as follows:

1_Person_Auto $=$ SOV_Auto_Drivers $*\left(\alpha_{1}+\beta_{1} *\right.$ Auto_Occupancy $)$

$$
\begin{gathered}
2 \text { Person_Auto }=\text { SOV_Auto_Drivers } *\left(\alpha_{2}+\beta_{2} * \text { Auto_Occupancy }\right) \\
+ \text { HOV-A }(2) \text { Drivers (for HBW only) } \\
3+\text { Person_Auto }=\text { SOV_Auto_Drivers } *\left(\alpha_{3}+\beta_{3} * \text { Auto_Occupancy }\right) \\
+ \text { HOV-B }(3+) \text { Drivers (for HBW only) }
\end{gathered}
$$

where $\alpha_{i}$ and $\beta_{i}$ vary based on purpose and on whether or not the auto occupancy is above or below 1.12.

An issue with the MWCOG model is with the manner in which adjustments are made to the choice probabilities. These adjustments are introduced in order to obtain target district-to-district mode shares and also to introduce some distance-based adjustments (e.g., if the highway distance is short, households with higher auto availability are more likely to drive than the logit model predicts). Calibrating a mode choice model to target aggregate shares is a common and accepted practice. However, the MWCOG model makes linear adjustments directly to the transit and carpool probabilities, and then manipulation is performed to assure that all probabilities are between 0 and 1 , and that they sum to 1 . This method of calibration applies linear adjustments to a non-linear model (the probabilities), and therefore the resulting probabilities are no longer logit. This means that all of the beneficial properties of logit (such as being consistent with behavioral theory and having known mathematical properties) are lost. The preferable method of calibrating to district-to-district shares is to apply the adjustments as constants directly in the utility equations. These additional constants can be calibrated such that district-to-district shares are achieved, and the resulting probabilities are logit probabilities.

## Mode Choice Analysis

Ideally, the best approach would be to estimate and employ a nested logit model (with a nest for the carpool alternative). It would also be desirable to predict utilization of the various transit modes instead of overall transit use. However, consistency with the MWCOG model and the small size of the panel survey necessitated the continued use of a two-stage Multinomial Logit model. The first set of models predicts the shares for the Drive Alone, Carpool and Transit modes. The second set of models predicts the shares of HOV2, HOV3 and HOV4+ drivers among the carpool trips.

The mode choice model is applied to 3 market segments only, which are based on vehicle ownership ( 0 vehicle, 1 vehicle and $2+$ vehicle households). In order to facilitate this, the output matrix from the trip distribution had to be split into the above market segments. This was achieved by using zone factors based on the number of 0 vehicle, 1 vehicle and $2+$ vehicle households for each zone.

The equations for the mode choice model and model coefficients were obtained from the MWCOG Travel Forecasting Model 2000 Calibration report. Some of the variables used to influence the shares are the peak and off-peak highway and transit skims, fares, operating costs, parking costs etc.

A second distinction in the TransForM model is that the choice of walk access to transit versus drive access to transit is predetermined based on the values of the walk and the drive access skim for each OD pair. If the generalized cost of the drive skim is less than the generalized cost of the walk skim for a particular OD pair, then drive access is preferred for that OD pair and the values of the drive access skims are used as the transit skim variable in the transit utility component. Likewise, the values of the walk access skims are used if the generalized cost of the walk access skim is less that that of the drive access skims. For OD pairs where walk or drive access to transit was infeasible, the transit choice was made unavailable.

Third, for the market segments with 0 vehicle households, the drive alone choice was made unavailable (unlike in the MWCOG model, which predicted shares for the drive alone alternative for 0 vehicle households).

Finally, the procedure of adjusting the shares to match observed shares and district to district totals was dispensed with. Instead, a calibration routine was developed at Caliper that adjusts the alternate specific coefficients of the individual utility components to match mode choice shares. This calibration routine was based on incremental logit coupled with a binary search algorithm to obtain the alternate specific constants. It must be noted that the calibration of the alternate specific constants was performed at regular intervals throughout the calibration effort, since the inputs to the mode choice (the congested highway travel times) were often updated during the calibration process. In order to match certain district to district shares, dummy variables were used in the utility equation and the coefficients of these dummy variables were adjusted to yield appropriate district to district shares. For instance, a dummy variable for transit was used in the DC sub-region and a positive co-efficient was calibrated to achieve the correct share of transit users in DC.

The outputs of the mode choice model consists of drive alone, HOV2 and HOV3+ trips for each of the purposes. The person trips obtained after applying the MNL models are converted to auto trips using auto occupancy rates derived from the COG 2000 panel survey. The auto occupancy rates for the 4+ carpool trips derived from the COG survey are $4.55,4.36,4.36$ and 4.38 for the HBW, HBS, HBO and NHB purposes respectively.

## Mode Choice Utility Specifications

The mode choice utilities for each of the purposes are shown in tables 5.1 and 5.2. Note that unless a distinction is made, the coefficients in the table below are the same for the three market segments. A description of the variables is then provided.

| Variable | Description | HBW | HBS | HBO | NHB |
| :--- | :--- | :---: | :---: | :---: | :---: |
| ASC_DA <br> 1 Veh HH | Alternate Specific Constant for the <br> Drive Alone mode for the 1 Veh HH <br> market segment | -0.178 | 3.389 | 1.387 | 1.907 |
| ASC_DA <br> $2+$ Veh HH | Alternate Specific Constant for the <br> Drive Alone mode for the 2+ Veh HH | 0.924 | 6.023 | 1.634 | 2.146 |


|  | market segment |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| ASC_CP <br> 0 Veh HH | Alternate Specific Constant for the Carpool mode for the 0 Veh HH market segment | -4.403 | 0.613 | -0.050 | 0.229 |
| $\begin{aligned} & \text { ASC_CP } \\ & 1 \text { Veh HH } \end{aligned}$ | Alternate Specific Constant for the Carpool mode for the 1 Veh HH market segment | -2.218 | 1.232 | -0.009 | 0.053 |
| $\begin{aligned} & \text { ASC_CP } \\ & 2+\text { Veh HH } \end{aligned}$ | Alternate Specific Constant for the Carpool mode for the $2+$ Veh HH market segment | -1.838 | 4.329 | 0.711 | 0.289 |
| LUMI\|Orig for DA | Land Use Mix Index at the origin zone for the Drive Alone mode only | -- | $2.267 \mathrm{e}-5$ | $2.585 \mathrm{e}-5$ | $1.369 \mathrm{e}-5$ |
| LUMI\|Dest for DA | Land Use Mix Index at the destination zone for the Drive Alone mode only | $-2.518 \mathrm{e}-5$ | $2.438 \mathrm{e}-5$ | $2.171 \mathrm{e}-5$ | $1.3 \mathrm{e}-5$ |
| LUMI\|Orig for Transit | Land Use Mix Index at the origin zone for the Transit mode only | $4.49 \mathrm{e}-5$ | -- | 5.194e-5 | -- |
| LUMI\|Dest for Transit | Land Use Mix Index at the destination zone for the Transit mode only | -- | $4.869 \mathrm{e}-5$ | $2.307 \mathrm{e}-5$ | $1.659 \mathrm{e}-5$ |
| Hwy IVTT | The Highway In-Vehicle Travel Time for the Drive Alone and the Carpool Modes | -0.03 | -0.00912 | -0.01902 | -0.03242 |
| Hwy Terminal Time | The Highway Terminal time for the Drive Alone and the Carpool Modes | -0.03 | -0.00912 | -0.01902 | -0.03242 |
| Hwy Oper Cost | The Highway Operating Cost based on 9.1 cents per mile for the Drive Alone and the Carpool Modes | -0.00425 | -0.00416 | -- | -- |
| Ln Hwy Oper Cost | The natural logarithm of the Highway Operating Cost for the Drive Alone and Carpool Modes | -- | -- | -0.78384 | -0.86043 |
| Hwy Park Cost | The Parking cost for the Drive Alone and the carpool modes | -0.00425 | -0.00416 | -- | -- |
| Hwy Toll | The Highway Toll for the Drive Alone and the carpool modes | -0.00425 | -0.00416 | -- | -- |
| Transit Walk Time | The Walk Access Time to transit for the walk to transit OD pairs (transit mode only) | -0.075 | -0.02432 | -0.04991 | -0.0286 |
| Transit Init Wait | The Initial Waiting Time for the transit mode | -0.075 | -0.02432 | -0.04991 | -0.06695 |
| Transit XFer Wait | The Transfer Waiting time for the transit mode | -0.075 | -0.02432 | -0.04991 | -0.06695 |
| Transit IVTT | The Transit In Vehicle Travel Time | -0.03 | -0.00912 | -0.01902 | -0.06695 |
| Transit Drive Access Time | The Transit Access Drive Time for the drive to transit OD pairs | -0.03 | -0.00912 | -0.01902 | -0.03242 |
| Transit Fare | The transit fare | -0.00425 | -0.00912 | -- | -- |
| Ln Transit Fare | The natural logarithm of the transit fare | -- | -- | -0.78384 | -0.86043 |
| Drive Bias for | The Drive Bias Dummy variable for the | -2.0499 | -2.9 | -2.9 | -1.4 |


| Transit 0 Veh <br> HH | transit mode only for the 0 Veh HH <br> market segment |  |  |  |  |
| :--- | :--- | :---: | :---: | :---: | :---: |
| Drive Bias for <br> Transit 1 Veh <br> HH | The Drive Bias Dummy variable for the <br> transit mode only for the 1 Veh HH <br> market segment | -0.5876 | 0.0 | -1.1 | 0.0 |
| Drive Bias for <br> Transit 2+ Veh <br> HH | The Drive Bias Dummy variable for the <br> transit mode only for the 2+ Veh HH <br> market segment | -0.3571 | 2.0 | -0.65 | 0.0 |
| Short Walk <br> Dummy | The Short Walk Dummy Variable for <br> the transit mode only. Applies if walk <br> to transit is the best option and if the <br> walk time is less than 5 minutes | -- | -- | 0.41346 | 0.76998 |
| Metrorail Bias <br> Dummy | The Metrorail Bias Dummy for the <br> transit mode only. Applies if Metrorail <br> IVTT is more than 25\% of Total IVTT | -- | 0.84404 | 0.69708 | 1.47447 |
| Carpool Hwy <br> Time Savings | The Highway Time Savings by using <br> Carpool (for the Carpool mode only) | 0.03611 | -- | -- | --- |
| Counties <br> Dummy for DC | The transit dummy variable for <br> households in the DC sub-region <br> (transit mode only) | 1.35 | -- | -- | -- |
| Counties <br> Dummy for <br> Inner Ring VA | The transit dummy variable for <br> households in the Inner Ring VA <br> subregion (transit mode only) | -1.9 | -- | -- | - |

Table 5.1 Utility Specification for the Basic Mode Choice MNL Model
Table 5.2 shows the utility specification for the MNL model that splits carpool trips into 2 occupancy, 3 occupancy and $4+$ occupancy trips. As in the previous specification, unless mentioned the coefficients apply to all the three market segments.

| Variable | Description | HBW | HBS | HBO | NHB |
| :--- | :--- | :---: | :---: | :---: | :---: |
| ASC_3 Persons <br> 0 Veh HH | Alternate Specific Constant for the <br> 3 Persons alternative for the 1 Veh <br> HH market segment | 0 | 0 | 0 | -0.92477 |
| ASC_3 Persons <br> 1 Veh HH | Alternate Specific Constant for the <br> 3 Persons alternative for the 2+ Veh <br> HH market segment | -1.47162 | -0.92201 | -0.31756 | 0 |
| ASC_3 Persons <br> 2+ Veh HH | Alternate Specific Constant for the <br> 3 Persons alternative for the 0 Veh <br> HH market segment | -1.88085 | -0.48966 | -0.15151 | 0 |
| ASC_4+ Persons <br> 0 Veh HH | Alternate Specific Constant for the <br> 4+ Persons alternative for the 1 Veh <br> HH market segment | 0 | 0 | 0 | -1.41003 |
| ASC_4+ Persons <br> 1 Veh HH | Alternate Specific Constant for the <br> 4+ Persons alternative for the 2+ Veh <br> HH market segment | -3.04973 | -1.51854 | 0 | 0 |
| ASC_4+ Persons <br> 2+ Veh HH | Alternate Specific Constant for the <br> 4+ Persons alternative for the 0 Veh <br> HH market segment | -2.54494 | -0.84071 | 0.21854 | 0 |
| HOV2 Oper Cost | The HOV2 highway operating cost <br> only for the 2 Person Alternative | -0.01124 | -- | -- | -- |


| HOV2 Toll | The HOV2 highway toll only for the <br> 2 Person Alternative | -0.05077 | -- | -- | -- |
| :--- | :--- | :---: | :---: | :---: | :---: |
| HOV2 Time | The HOV2 highway time only for the <br> 2 Person Alternative | -- | -0.45633 | -0.6853 | -0.00709 |
| HOV2 Distance | The HOV2 highway distance only <br> for the 2 Person Alternative | -- | -- | -- | -0.00187 |
| HOV3+ Oper <br> Cost | The HOV3+ highway operating cost <br> only for the 3 and 4+ Persons <br> Alternatives | -0.01124 | -- | -- | -- |
| HOV3+ Toll | The HOV3+ highway toll only for <br> the 3 and 4+ Persons Alternatives | -0.05077 | -- | -- | -- |
| HOV3+ Time | The HOV3+ highway time only for <br> the 3 and 4+ Persons Alternatives | -- | -0.45633 | -0.6853 | -0.00709 |
| HOV3+ Distance | The HOV3+ highway distance only <br> for the 3 and 4+ Persons Alternatives | -- | -- | -- | -0.00187 |

Table 5.2 Utility Specification for the Carpool MNL Model
In the above tables, it must be noted that the peak highway skims are used for the HBW trip purpose, whereas the off peak skims are used for all the other modes. The HOV skims are used in the second MNL model that splits the carpool trips into 2, 3 and 4+ occupancy trips. The skim values for the highway modes are expressed in tenths of miles and minutes in the above equations.

For the transit trips, depending on whether walk or drive is likely (based on the total generalized cost), the walk skim times or the drive skim times are used.

The Land Use Mix Index is a variable used in the above specification and is defined as


HH_POPD = Household population density and N_EMPD = Normalized employment density

Further, the utility specification for the main MNL model includes sub-region dummy matrices for the transit alternative in the HBW purpose (for the DC and the Inner Ring VA sub-regions). This variable was introduced to capture the relatively higher use of transit in the DC sub-region for the work purpose.

## Mode Choice Results

The mode choice results for each of the purposes are shown in Tables 5.3 to 5.6. The percentages of the individual modes are indicated in brackets.

| Market <br> Segment | Drive <br> Alone | Carpool | Transit | 2_Person <br> CP | 3_Person <br> CP | 4+_Person <br> CP |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 Veh HH | 0 <br> $(0.0 \%)$ | 90,986 <br> $(31.7 \%)$ | 196,339 <br> $(68.3 \%)$ | 30,377 | 30,304 | 30,304 |
| 1 Veh HH | 758,120 <br> $(67.4 \%)$ | 106,418 <br> $(9.5 \%)$ | 260,298 <br> $(23.1 \%)$ | 83,400 | 19,081 | 3,937 |
| 2+Veh HH | $2,029,059$ <br> $(86.6 \%)$ | 138,359 <br> $(5.9 \%)$ | 176,141 <br> $(7.5 \%)$ | 112,445 | 17,107 | 8,806 |
| All Segments | $\mathbf{2 , 7 8 7 , 1 8 0}$ <br> $(\mathbf{7 4 . 2 \%})$ | $\mathbf{3 3 5 , 7 6 5}$ <br> $(\mathbf{8 . 9 \%})$ | $\mathbf{6 3 2 , 7 7 9}$ <br> $(\mathbf{1 6 . 8 \%})$ | $\mathbf{2 2 6 , 2 2 3}$ | $\mathbf{6 6 , 4 9 3}$ | $\mathbf{4 3 , 0 4 8}$ |

Table 5.3 HBW Mode Choice Shares

| Market <br> Segment | Drive <br> Alone | Carpool | Transit | 2_Person <br> CP | 3_Person <br> CP | 4+_Person <br> CP |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 Veh HH | 0 <br> $(0.0 \%)$ | 193,171 <br> $(91.4 \%)$ | 18,121 <br> $(8.6 \%)$ | 64,389 | 64,390 | 64,390 |
| 1 Veh HH | 704,523 <br> $(89.4 \%)$ | 73,460 <br> $(9.3 \%)$ | 10,042 <br> $(1.3 \%)$ | 45,436 | 18,071 | 9,952 |
| 2+Veh HH | $1,353,583$ <br> $(85.0 \%)$ | 233,925 <br> $(14.7 \%)$ | 5,516 <br> $(0.3 \%)$ | 114,427 | 70,130 | 49,368 |
| All Segments | $\mathbf{2 , 0 5 8 , 1 0 6}$ <br> $(\mathbf{7 9 . 4 \%})$ | $\mathbf{5 0 0 , 5 5 7}$ <br> $(\mathbf{1 9 . 3 \%})$ | $\mathbf{3 3 , 6 8 0}$ <br> $\mathbf{( 1 . 3 \% )}$ | $\mathbf{2 2 4 , 2 5 3}$ | $\mathbf{1 5 2 , 5 9 2}$ | $\mathbf{1 2 3 , 7 1 1}$ |

Table 5.4 HBS Mode Choice Shares

| Market <br> Segment | Drive <br> Alone | Carpool | Transit | 2_Person <br> CP | 3_Person <br> CP | 4+_Person <br> CP |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 Veh HH | 0 <br> $(0.0 \%)$ | 519,124 <br> $(91.2 \%)$ | 50,378 <br> $(8.8 \%)$ | 173,027 | 173,048 | 173,048 |
| 1 Veh HH | $1,701,007$ <br> $(79.5 \%)$ | 390,967 <br> $(18.3 \%)$ | 48,664 <br> $(2.3 \%)$ | 143,282 | 104,342 | 143,342 |
| 2+Veh HH | $3,138,855$ <br> $(71.8 \%)$ | $1,181,756$ <br> $(27.0 \%)$ | 53,409 <br> $(1.2 \%)$ | 380,605 | 327,293 | 473,857 |
| All Segments | $\mathbf{4 , 8 3 9 , 8 6 3}$ <br> $(\mathbf{6 8 . 3 \%})$ | $\mathbf{2 , 0 9 1 , 8 4 8}$ <br> $\mathbf{( 2 9 . 5 \% )}$ | $\mathbf{1 5 2 , 4 5 2}$ <br> $(\mathbf{2 . 2 \%})$ | $\mathbf{6 9 6 , 9 1 5}$ | $\mathbf{6 0 4 , 6 8 4}$ | $\mathbf{7 9 0 , 2 4 8}$ |

Table 5.5 HBO Mode Choice Shares

| Market <br> Segment | Drive <br> Alone | Carpool | Transit | 2_Person <br> CP | 3_Person <br> CP | 4+_Person <br> CP |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 Veh HH | 0 <br> $(0.0 \%)$ | 397,169 <br> $(90.4 \%)$ | 42,403 <br> $(9.6 \%)$ | 237,490 | 94,194 | 65,484 |
| 1 Veh HH | $1,263,876$ <br> $(85.0 \%)$ | 185,346 <br> $(12.5 \%)$ | 37,046 <br> $(2.5 \%)$ | 59,190 | 59,190 | 66,965 |
| 2+Veh HH | $2,230,173$ <br> $(85.8 \%)$ | 335,741 <br> $(12.9 \%)$ | 33,652 <br> $(1.3 \%)$ | 106,243 | 106,242 | 123,254 |
| All Segments | $\mathbf{3 , 4 9 4 , 0 4 9}$ <br> $(\mathbf{7 7 . 2 \%})$ | $\mathbf{9 1 8 , 2 5 6}$ <br> $(\mathbf{2 0 . 3 \% )}$ | $\mathbf{1 1 3 , 1 0 2}$ <br> $(\mathbf{2 . 5 \%})$ | $\mathbf{4 0 2 , 9 2 4}$ | $\mathbf{2 5 9 , 6 2 7}$ | $\mathbf{2 5 5 , 7 0 4}$ |

Table 5.6 NHB Mode Choice Shares

Table 5.7 shows the estimated versus the observed mode choice shares. The observed mode choice shares are from the COG 2000, Version 2.1D Draft \#50 report. In the table below, the auto occupancy factor is defined as the ratio of the share of the non-transit trips (that is the sum of shares of drive alone and carpool trips) to the share of drive alone trips.

| Purpose | Observed <br> Transit Pct | Estimated <br> Transit Pct | Observed <br> Auto-Occ Factor | Estimated <br> Auto-Occ Factor |
| :---: | :---: | :---: | :---: | :---: |
| HBW | 16.9 | 16.8 | 1.12 | 1.12 |
| HBS | 1.2 | 1.3 | 1.23 | 1.24 |
| HBO | 2.1 | 2.2 | 1.44 | 1.43 |
| NHB | 2.5 | 2.5 | 1.25 | 1.26 |

Table 5.7 Estimated versus Observed Mode Choice Shares

## HBW Mode Choice Analysis

Since a significant percent of the HBW trips are transit trips, the HBW shares are analyzed in greater detail below.

The mode choice results for the HBW purpose are listed by jurisdiction in Table 5.8 and 5.9. Table 5.8 shows the shares of trips originating from the jurisdiction, whereas table 5.9 shows the shares of trips destined to the jurisdiction.

| Jurisdiction | Drive Alone <br> Pct | Transit <br> Pct | Carpool <br> Pct |
| :--- | :---: | :---: | :---: |
| District of Columbia DC | 25.9 | 68.9 | 5.2 |
| Montgomery MD | 65.8 | 26.8 | 7.4 |
| Prince George's MD | 64.4 | 27.1 | 8.5 |
| Arlington VA | 54.9 | 37.0 | 8.1 |
| Alexandria VA | 59.2 | 31.9 | 8.9 |
| Fairfax and Falls Church VA | 84.4 | 5.8 | 9.8 |
| Loudoun VA | 89.6 | 0.6 | 9.8 |
| Manassas \& PrinceWilliam VA | 84.1 | 6.3 | 9.6 |
| Frederick MD | 88.0 | 0.0 | 12.0 |
| Howard MD | 83.5 | 6.8 | 9.7 |
| Anne Arundel MD | 86.0 | 2.0 | 12.0 |
| Charles MD | 87.0 | 1.4 | 11.6 |
| Carroll MD | 88.9 | 0.0 | 11.1 |
| Calvert MD | 87.8 | 1.8 | 10.4 |
| St. Mary's MD | 87.6 | 0.0 | 12.4 |
| King George VA | 88.2 | 0.0 | 11.8 |
| Fredericksburg VA | 79.2 | 0.0 | 20.8 |
| Stafford VA | 90.3 | 0.4 | 9.3 |
| Spotsylvania VA | 89.7 | 0.0 | 10.3 |
| Fauquier VA | 88.7 | 0.0 | 11.3 |
| Clarke VA | 87.1 | 0.0 | 12.9 |
| Jefferson WV | 86.2 | 0.0 | 13.8 |

Table 5.8 HBW Mode Choice Shares by Jurisdiction (Origin)

| Jurisdiction | Drive Alone <br> Pct | Transit <br> Pct | Carpool <br> Pct |
| :--- | :---: | :---: | :---: |
| District of Columbia DC | 48.8 | 45.5 | 5.7 |
| Montgomery MD | 70.5 | 21.7 | 7.9 |
| Prince George's MD | 75.2 | 15.1 | 9.7 |
| Arlington VA | 66.7 | 23.6 | 9.7 |
| Alexandria VA | 73.1 | 17.9 | 9.0 |
| Fairfax and Falls Church VA | 85.1 | 4.6 | 10.2 |
| Loudoun VA | 89.4 | 0.2 | 10.4 |
| Manassas \& PrinceWilliam VA | 87.4 | 3.1 | 9.6 |
| Frederick MD | 87.9 | 0.0 | 12.1 |
| Howard MD | 87.7 | 2.7 | 9.6 |
| Anne Arundel MD | 88.4 | 0.2 | 11.5 |
| Charles MD | 87.3 | 0.3 | 12.4 |
| Carroll MD | 91.0 | 0.0 | 9.0 |
| Calvert MD | 89.0 | 0.0 | 11.0 |
| St. Mary's MD | 87.7 | 0.0 | 12.3 |
| King George VA | 88.5 | 0.0 | 11.5 |
| Fredericksburg VA | 87.1 | 0.0 | 12.9 |
| Stafford VA | 89.7 | 0.0 | 10.3 |
| Spotsylvania VA | 89.1 | 0.0 | 10.9 |
| Fauquier VA | 89.2 | 0.0 | 10.8 |
| Clarke VA | 90.1 | 0.0 | 9.9 |
| Jefferson WV | 87.9 | 0.0 | 12.1 |

Table 5.9 HBW Mode Choice Shares by Jurisdiction (Destination)
In addition to the above shares, the sub-region to sub-region transit trips were observed. To facilitate this, the entire region is divided into 6 sub-regions as shown in Figure 6.1. The six sub-regions for study are:

1. DC
2. Montgomery, MD
3. Prince Georges County, MD
4. Inner Ring VA Counties (Fairfax, Fairfax City, Arlington, Alexandria)
5. Outer Ring VA Counties
6. Outer Ring MD Counties

The sub-regions are based on the density of transit routes in the region. For example, the transit coverage is the greatest in the DC sub-region and transit coverage in the outer ring counties is not as high.


Figure 5.1 Sub-regions for Mode Choice Analysis
The CTPP Part 3 data from the 2000 census was used to determine the percentage of transit flows from sub-region to sub-region using the above classification. The model was then calibrated (with the help of dummy variables to capture transit flows in DC and Inner Ring VA) to approximate the trend of transit flow trips depicted by CTPP Part 3 data.

Tables 5.10 to 5.12 show the regional matrices depicting the percentages of auto and transit trips (for the HBW purpose). Note that the sum of the three matrices yields 100 percent in all the cells.

|  | DC | Montgo- <br> mery | PGC | Inner Ring <br> VA | Outer Ring <br> VA | Outer Ring <br> MD |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| DC | 21.8 | 25.0 | 32.7 | 34.7 | 43.6 | 48.3 |
| Montgomery | 45.6 | 69.9 | 74.6 | 78.3 | 82.8 | 85.2 |
| PGC | 46.7 | 66.1 | 75.0 | 73.3 | 76.9 | 83.3 |
| Inner Ring <br> VA | 58.2 | 77.6 | 79.8 | 81.4 | 87.2 | 84.3 |
| Outer Ring <br> VA | 69.3 | 85.8 | 86.8 | 87.2 | 88.5 | 90.7 |
| Outer Ring <br> MD | 79.3 | 86.8 | 86.9 | 87.0 | 88.1 | 88.7 |

Table 5.10 HBW Drive Alone Sub-region to Sub-region Shares

|  | DC | Montgo- <br> mery | PGC | Inner Ring <br> VA | Outer Ring <br> VA | Outer Ring <br> MD |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| DC | 75.0 | 71.2 | 58.6 | 57.4 | 32.8 | 22.8 |
| Montgomery | 49.9 | 22.8 | 16.8 | 11.2 | 6.5 | 2.6 |
| PGC | 48.1 | 25.7 | 15.3 | 15.0 | 9.8 | 3.4 |
| Inner Ring <br> VA | 34.9 | 13.1 | 9.2 | 8.6 | 2.5 | 3.2 |
| Outer Ring <br> VA | 22.0 | 5.3 | 4.1 | 3.0 | 1.2 | 0.3 |
| Outer Ring <br> MD | 11.3 | 3.1 | 2.8 | 2.4 | 0.1 | 0.3 |

Table 5.11 HBW Transit Sub-region to Sub-region Shares

|  | DC | Montgo- <br> mery | PGC | Inner Ring <br> VA | Outer Ring <br> VA | Outer Ring <br> MD |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| DC | 3.2 | 3.8 | 8.7 | 7.9 | 23.6 | 28.9 |
| Montgomery | 4.5 | 7.3 | 8.6 | 10.5 | 10.7 | 12.2 |
| PGC | 5.2 | 8.2 | 9.7 | 11.7 | 13.3 | 13.3 |
| Inner Ring <br> VA | 6.9 | 9.3 | 11.0 | 10.0 | 10.3 | 12.5 |
| Outer Ring <br> VA | 8.7 | 8.9 | 9.1 | 9.81 | 10.3 | 9.0 |
| Outer Ring <br> MD | 9.4 | 10.1 | 10.3 | 10.6 | 11.8 | 11.0 |

Table 5.12 HBW Carpool Sub-region to Sub-region Shares
The above three matrices illustrate a few points. For instance, the transit share is higher in the DC sub-region (both flows into DC and out of DC). This is according to expectations, since the transit coverage is most dense around this sub-region. The outer ring counties have fewer transit trips.

Figure 5.2 shows a pie-chart depicting the mode choice shares of trips originating in each sub-region. Figure 5.3 shows a pie-chart depicting the mode choice shares of trips terminating in each sub-region. Again, these graphs show a higher transit percentage for the DC sub-region.


Figure 5.2 Mode Shares of HBW Trips - Origin


Figure 5.3 Mode Shares of HBW Trips - Destination

Table 5.13 shows the final auto trips for each purpose for the LOV, HOV2, HOV3+ modes and also shows the final transit person trips.

| Purpose | LOV Trips | HOV2 Trips | HOV3+ Trips | Transit Trips <br> (Person) |
| :--- | :---: | :---: | :---: | :---: |
| HBW | $2,787,180$ | 113,112 | 31,626 | 632,779 |
| HBS | $2,058,106$ | 112,127 | 79,238 | 33,680 |
| HBO | $4,839,863$ | 348,458 | 382,811 | 152,452 |
| NHB | $3,494,049$ | 201,462 | 144,923 | 113,102 |

Table 5.13 Final Trips by Mode

## Chapter 6 Time of Day Trip Allocation

This chapter describes the procedure used to allocate trips by time of day. The time of day routine generates AM, PM and Off-peak trip matrices from the output mode choice matrices. The procedure is performed in two parts-- first for miscellaneous trips (trips that are not modeled but are input to the model) and second for modeled trips. The procedure closely follows the MWCOG model, except that the time of day percentages for modeled trips were computed from the COG 2000 Panel Survey.

After the time of day factors are applied to obtain OD matrices by time period, the modeled trip matrices for each purpose are summed up by mode (SOV, HOV2, HOV3+) in preparation for traffic assignment. (In the TransForM model formulation, these modes are given as LOV, AUTO2 and AUTO3. )

## Miscellaneous Trips Time of Day

The miscellaneous trips consist of additional matrices that are not modeled explicitly but are inputs to the TransForM model. The trips come from the following trip classes:

- External to External Auto Trips
- External to External Truck Trips
- Taxi Trips
- Visitor Trips
- School Trips
- Airport Trips

An input matrix with the above vehicle trips was used along with a time of day lookup table to generate miscellaneous trips for the three periods. The lookup table for the miscellaneous trips contains the percentage of trips for each of the above classes for each of the time periods. Additionally, the lookup table also consists of the factors to split the medium and heavy truck trips (that are modeled) into the three periods. The factors are shown in Table 6.1. These factors are from the MWCOG model.

| Classification | AM Percent | PM Percent | OP Percent |
| :--- | :---: | :---: | :---: |
| Medium Truck Trips | 19.5 | 15.2 | 63.3 |
| Heavy Truck Trips | 15.4 | 13.0 | 71.6 |
| External Auto Trips | 18.0 | 22.0 | 60.0 |
| External Truck Trips | 23.0 | 11.0 | 66.0 |
| Taxi Trips | 9.0 | 27.0 | 64.0 |
| Visitor Trips | 33.0 | 33.0 | 34.0 |
| School Trips | 33.0 | 33.0 | 34.0 |
| Airport Trips | 10.0 | 10.0 | 80.0 |

Table 6.1 Miscellaneous Time of Day Factors

For the external to external auto trips, the trips were further split into SOV, HOV2 and HOV3+ trips using the percentage splits of $50.21,34.26$ and 11.53 percent respectively.

## Time of Day Allocation of Modeled Trips

This component of the time of day procedure splits the HBW, HBS, HBO and NHB trips by time period using departure and return percentages. The departure and return percentages were computed from the COG 2000 Panel Survey.

It was observed from the survey data (from Figure 6.1 below) that the time periods that appropriately classify the AM, PM and Off-peak periods were:

- AM - 6 AM to 9 AM
- PM - 4 PM to 6:30 PM
- Off-peak - Remaining hours

Time of Day Distribution from Survey


Figure 6.1 Time of Day Distribution from the COG 2000 Panel Survey
During the calibration process, several other configurations were examined to classify the periods. For instance, we tried MWCOG's recommendation that the PM peak be from 4 PM to 7PM. However, based on excessive flows in the PM period, we reduced the PM peak by 30 minutes (as is evident from the above graph).

The time of day factors are shown in Table 6.2. Note that each column constitutes 50 percent of the total trips for that purpose.

| Period | HBW <br> Dep | HBW <br> Return | HBS <br> Dep | HBS <br> Return | HBO <br> Dep | HBO <br> Return | NHB <br> Dep | NHB <br> Return |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AM | 33.3 | 1.0 | 4.8 | 1.2 | 11.1 | 3.3 | 2.9 | 2.9 |
| PM | 1.3 | 28.5 | 37.8 | 34.0 | 30.6 | 35.8 | 39.1 | 39.1 |
| OP | 15.4 | 20.5 | 7.4 | 14.8 | 8.3 | 10.9 | 8.0 | 8.0 |

Table 6.2 Time of Day Departure and Return Percentages
The time of day procedure takes as input the SOV, HOV2 and HOV3+ trips for each purpose (from the mode choice results). The procedure then split the trips for each purpose using the above factors into the three time periods to produce departure and return trips for each purpose and mode. The return trips matrices are then transposed. Finally the AM, PM and OP trips by mode (SOV, HOV2 and HOV3+) are generated by summing the appropriate matrices. For example:

HBW AM SOV Trips $=0.333^{*}($ HBW SOV Trips $)+0.01^{*}($ Transposed HBW SOV Trips)

Finally, the modeled trips are combined with the miscellaneous trips to yield the final trip matrices (by time period and mode) for the assignment process.

## Time of Day Results

Table 6.3 shows the final trip totals (both modeled and miscellaneous trips) from the Time of Day Analysis.

| Mode/Period | AM | PM | OP | Total |
| :--- | :---: | :---: | :---: | :---: |
| SOV | $2,182,533$ | $2,977,341$ | $8,650,250$ | $13,810,123$ |
| HOV2 | 261,265 | 356,997 | 811,548 | $1,429,810$ |
| HOV3+ | 79,785 | 124,519 | 438,987 | 643,291 |
| Truck | 81,147 | 62,113 | 285,075 | 428,335 |
| Airport | 2,566 | 2,566 | 20,528 | 25,660 |
| Total | $\mathbf{2 , 6 0 7 , 2 9 5}$ | $\mathbf{3 , 5 2 3 , 5 3 5}$ | $\mathbf{1 0 , 2 0 6 , 3 8 8}$ | $\mathbf{1 6 , 3 3 7 , 2 1 8}$ |

Table 6.3 Time of Day Results

## Chapter 7 Traffic Assignment

This chapter discusses the highway traffic assignment procedure and presents the assignment results. The major differences in the assignment methodology from that used in the MWCOG model are noted throughout the discussion. The model results are then described in detail. The results presented are the result of feedback loops run with the full model.

## Traffic Assignment Inputs and Volume Delay Functions

In TransForM, the TransCAD Multi-Modal, Multi-class (MMA)User Equilibrium highway assignment procedure is run for each of the three time periods (AM, PM and $\mathrm{OP})$. The primary inputs used in the assignment procedure are:

- Link travel times and capacities (from the appropriate peak or off-peak network).
- Link volume-delay function (VDF) parameters.
- Trip tables matrices for each time period by highway user class.

The AM and the PM assignments use the peak network whereas the OP assignment uses the off-peak network. The link free flow travel times and link capacities are obtained from the appropriate network file.

Five classes of traffic are assigned simultaneously. These are single occupant vehicles, HOV-2, HOV-3, trucks, and airport vehicle trips. The trip matrices are obtained after the time of day procedure and consist of 5 classes for each time period.

In addition to the above inputs, traffic assignment routines require the use of a delay function that computes congested link travel times depending on the flow of traffic on the link.

In the MWCOG model, which employs TP+, the volume-delay functions are specified as piecewise linear functions. Furthermore, an upper bound or ceiling is applied to the resulting congested travel times. If a volume-to-capacity ratio (v/c) exceeds the highest value specified in the volume delay function, the travel time no longer increases. In the Version 2.1 C of the MWCOG model that we initially reviewed and seemingly in Version 2.1 Draft \#50, this cutoff in travel time occurs at a v/c ratio of 1.5. This practice is inconsistent with traffic assignment theory and also impairs the convergence of the MWCOG traffic assignment model.

In the TransForM model, we sought an improved approach, and we chose to use the BPR volume delay functions that are recommended for planning applications in the 2000 Highway Capacity Manual. Other choices would certainly be possible, but would require additional data that was not available. The form of the BPR functions is shown below:

$$
t=t_{f}\left[1+\alpha\left(\frac{v}{c}\right)^{\beta}\right]
$$

where t is the congested travel time
$t_{f}$ is the free flow travel time
$\alpha$ is the BPR alpha parameter
$\beta$ is the BPR beta parameter
$v$ is the volume on the link
$c$ is the link capacity
The values of the link VDF parameters are defined based on the functional class of the link and are shown in Table 7.1. The corresponding delay curves are shown in Figure 7.1 below. The initial parameter values ( $\alpha$ and $\beta$ ) were obtained from recommendations in the Highway Capacity Manual. Subsequently, some of the parameters were altered during the calibration process. The adjustment was based on the percent difference between the flows and the counts by functional class. These values along with the speeds and lane capacities are obtained from speed and capacity lookup tables input to the model.

| Functional Class | BPR Alpha | BPR Beta |
| :--- | :---: | :---: |
| Freeways | 0.25 | 9.0 |
| Expressways | 0.75 | 9.0 |
| Major Arterials | 1.25 | 6.0 |
| Minor Arterials | 1.00 | 5.0 |
| Collectors | 1.00 | 5.0 |
| Ramps | 1.00 | 6.0 |
| Centroid Connectors | 0.15 | 4.0 |

Table 7.1 BPR Link Delay Parameters


Figure 7.1 Volume Delay Functions based on Functional Class

## Traffic Assignment Procedure

A multi-modal, multi-class assignment (MMA) is performed to assign the 5 classes. Three assignment routines are executed with one for each time period. The assignment employs exclusion sets for the vehicle classes to exclude the facilities that are prohibited for each vehicle class. This is achieved in TransCAD by disabling link segments by direction depending on the vehicle class. In addition, reversible lanes in the AM and the PM periods are incorporated using TransCAD's network update functions.

During the calibration process, the assignment procedures were run until convergence with a relative gap of 0.001 . This is a much lower value than is typically encountered in U.S. modeling practice. It was observed that the number of iterations required to achieve this tolerance was somewhere between 80 to160 depending upon the time period and the level of congestion. The number of feedback loops varied depending upon the changes that were made in calibration.

The "relative gap" in TransCAD is a metric that has been shown in the literature to be one of the more useful and rigorous measures of convergence to user equilibrium. It is the same measure that is used in EMME/2. However, it is a very different and significantly more stringent measure than the measure that is named the "gap" in TP+.

The gap measure in TP+ is a measure of the difference between successive iterations of the assignment and is calculated as $\left(\mathrm{VHT}_{\mathrm{i}}-\mathrm{VHT}_{\mathrm{i}-1}\right) / \mathrm{VHT}_{\mathrm{i}}$. This is a very poor measure of convergence which overstates the actual convergence and gives improper signals to modelers to use too fewer iterations. Limiting the v/c ratio further reduces the gap although it does so artificially. This exacerbates the problem of premature termination of the traffic assignment.

While 0.001 was used in calibration, we also set about determining the appropriate level of convergence to be used in model application. This is useful, because the selection of the gap and the feedback convergence criteria have a dominant impact upon model run times. We determined that for most purposes, a true relative gap of 0.01 in the traffic assignment would be sufficient. In the model, it takes 24-26 iterations to converge to below this value for the AM peak period, 18-21 iterations for the Off-peak period, and 46-50 iterations for the PM peak.

## Assignment Procedure with Feedback

The TransForM model is run sequentially with feedback loops to get a more consistent estimate of traffic flows and transit utilization. In the first loop, all model steps are executed, and in subsequent loops, all steps except those prior to skimming and trip distribution are run again. One loop of the feedback consists of the highway skimming procedure (with the latest estimate of the congested times), trip distribution, mode choice procedure, time of day and traffic assignment procedures. During each feedback loop and after the assignment stage, TransCAD uses the Method of Successive Averages (MSA) to generate new estimates of the MSA link flows and the corresponding travel times for each of the three assignments (AM, PM and OP). These estimates are internally stored in the peak and Off-Peak network files. The MSA flows and times for the AM and the PM assignments are stored in the peak network file, whereas the MSA flows and times for the OP period are stored in the Off-Peak network.

After each run of the feedback loop, these MSA flow and time vectors are extracted from the respective network files. A weighted average of the AM and PM MSA times (using the AM and PM MSA flows as weights) is constructed to represent the peak congested link travel times, which are input into the skimming stage of the next feedback loop. Similarly, the off peak congested times are simply the OP MSA times extracted from the offpeak network.

Four loops of the model are typically executed. At the end of the feedback procedure, the final congested link flows and times are written out. With the current version of the model, a high degree of feedback convergence was also obtained. In particular, the differences between the final input skim matrix and the one that is consistent with the final solution has a root-mean-square error of less than one percent.

Another refinement in the calculation of the model to feedback convergence is the use of congested times for the initial skims. These were derived from prior model runs
generated during the calibration effort and lead to a more highly converged, consistent, and fast running model.

The model with feedback takes approximately 6 hours to run on 2.1 GHz Core Duo Dell laptop. One loop of the model takes 1.5 hours on that machine. The model takes 1hour per loop and a total of 4 hours to run on a HP 2.2 GHz Dual Opteron 275 with two cores per chip. These relatively fast times are due in some measure to the multi-threaded traffic assignment in TransCAD. Further speed improvements are achievable with distributed processing and through several other optimizations that we have yet to apply.

## Traffic Assignment Results

The results of the traffic assignment are presented in this section. Traffic flow maps for the entire region and for Prince George's County are provided to give an overview of regional traffic levels and their geographic distribution. A comparison of the model outputs with traffic counts is then provided using the root mean square error (RMSE) as a measure of how well the model fits ground counts. These RMSE values are provided for each time period and are for each county and functional class combination. The annual estimated Year 2000 VMT values from the model are presented by county and compared to the estimates from HPMS. Screenline analyses are also presented. Potential improvements to the model are suggested, wherever appropriate.

In viewing the results, it should be kept in mind that a principal focus was on achieving a closer calibration for the more detailed Prince Georges County network. This has been achieved, while at the same time the model fits well for other jurisdictions and the region as a whole. Undoubtedly, the model and the model calibration could be further improved by a similar focusing of attention on other counties and the District of Columbia. This would include adding links to the road network as well as revising centroid connectors. Also, we suspect that the development and application of some adjustment factors for county to county flows, survey underreporting, and data problems with traffic counts would result in a superior model. It is, however, encouraging that the model has relatively high goodness of fit without such adjustments.

In judging the comparisons with traffic counts and HPMS data, we suggest that a measure of caution is warranted. In our experience and in that of the MWCOG, there are many errors in published traffic counts, and these errors are a major impediment to the development of improved regional and local models. Our understanding is that MWCOG will attempt to remedy this situation in the future.

In the remainder of this chapter, we present the results of the traffic assignments after the full feedback process. Figure 7.2 shows the flow pattern of the daily flows for the entire region. Figures 7.3 to 7.5 show the flow patterns of the daily, AM and PM flows for the for an area containing the Beltway, the District of Columbia and the Prince George's County

In these figures, the width of the road segments depends on the modeled flow value, while the colors reflect the modeled volume to capacity (V/C) ratios. Links with a low V/C value (and hence less congested) are coded in green, whereas links with high V/C values are coded in red. Yellow and orange colors depict links within these extremes. The maps show that flow is generally higher into the DC region during the AM period and the reverse in the PM period. The PM peak period is generally more congested than the AM peak period.

The estimated Year 2000 regional vehicle miles of travel from the TransForM model is $154,409,000$. This is comparable but somewhat higher than the corresponding estimate of $143,644,783$ from the Version 2.1D, \#50 forecast run of the MWCOG model.

| Region | Daily VMT (in thousands) |
| :---: | :---: |
| TOTAL | 154,409 |
| DC | 8,214 |
| VA | 57,412 |
| MD | 85,502 |

Table 7.2 Regional VMT Estimates


Figure 7.2 Daily Regional Traffic Flow Map


Figure 7.3 Daily Traffic Flows: Beltway, DC and Prince George’s County


Figure 7.4 AM Traffic Flows: Beltway, DC and Prince George's County


Figure 7.5 PM Traffic Flows: Beltway, DC and Prince George's County

## Comparison of Model Output with Traffic Counts--RMSE Estimates

The model output was compared with available traffic counts. This is done using the root mean square statistic applied to the links for which counts are available.

The counts for comparison were obtained from the Prince George's County Planning Department, the Maryland and Virginia DOTs, and the District Department of Transportation. Counts by time period and direction were always sought, but not always available. The details of the count databases were discussed under the network building chapter (Chapter 2).

The Percent RMSE formula is given by:
$\% R M S E=100 * \sqrt{\left(\sum_{i}(\text { Model }- \text { Count })^{2} /(\text { Numberofcaints }-1)\right.} /\left(\sum_{i}(\right.$ Count $/$ NumberofCaints $)$
Table 8.3 shows the Daily RMSE statistics by county and functional class. In the tables below, it should be noted that RMSE statistics where the number of observations is 15 or below may not be meaningful and representative for that classification. Also, for the counties not listed, the count information was not available. Furthermore, it is obvious to us and also to MWCOG that not all published counts are correct.

The Daily Regional RMSE value for all the observations is 41.69 percent. The RMSE for freeways is 15.52 percent, which meets the acceptable standard of 20 percent or less. The results for Prince George's County were even better with an overall RSME of 35.74 and a value of 12.69 percent for freeways. The results for Montgomery County are also especially good with an overall RSME of 25.19 percent and 15.67 percent for freeways.

As regards other counties, it is to be noted that no additional calibration for these counties was performed. It is observed that the RMSE values for Montgomery County are acceptable and the model fits the counts well in Montgomery. However, for DC and Fairfax, the model warrants improvement as the RMSE's are higher than desired. In particular, it seems that the flows are higher in DC (by about 16 percent) and lower in Fairfax in comparison to the counts (by about 9 percent). A general observation from the flow maps is that this may be caused by a slightly high number of trips from Fairfax to DC (via the bridges). This could be due a variety of reasons that are not captured in the model currently but may be improved significantly with additional data. Nevertheless, the model fits the overall counts well, and the RMSE values on the major roads are well within the acceptable limits.

The RMSE values are provided by time period in Tables 7.4 to 7.6 that follow. Again, the RMSE for each time period for the overall network is acceptable and somewhat better for Prince George's County. Again, similar observations in the daily RMSE's can be made at the county level. In general, the RMSE values by time period tend to be higher than for daily values, as is reflected in the tables.

| County | FClass | Pct RMSE | Num Obs | TotalFlow | TotalCount | Pct Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ALL Counties | ALL Classes | 41.33 | 2086 | 28,339,640 | 27,817,345 | 1.88 |
| ALL Counties | Freeways | 15.42 | 105 | 7,334,492 | 7,508,328 | -2.32 |
| ALL Counties | Major Arterials | 43.59 | 718 | 12,071,226 | 11,011,901 | 9.62 |
| ALL Counties | Minor Arterials | 53.30 | 671 | 4,484,348 | 4,324,570 | 3.69 |
| ALL Counties | Collectors | 89.49 | 411 | 1,015,660 | 1,407,578 | -27.84 |
| ALL Counties | Expressways | 26.52 | 125 | 3,230,283 | 3,256,646 | -0.81 |
| ALL Counties | Ramps | 60.47 | 36 | 199,584 | 227,187 | -12.15 |
| PG County | ALL Classes | 35.45 | 992 | 12,577,597 | 12,423,373 | 1.24 |
| PG County | Freeways | 12.67 | 46 | 3,431,988 | 3,497,144 | -1.86 |
| PG County | Major Arterials | 37.46 | 238 | 3,397,700 | 3,153,986 | 7.73 |
| PG County | Minor Arterials | 52.37 | 468 | 2,945,139 | 2,761,817 | 6.64 |
| PG County | Collectors | 72.03 | 118 | 232,675 | 336,000 | -30.75 |
| PG County | Expressways | 25.69 | 97 | 2,429,552 | 2,506,410 | -3.07 |
| PG County | Ramps | 36.23 | 17 | 136,495 | 135,566 | 0.69 |
| Montgomery | ALL Classes | 23.83 | 80 | 2,725,916 | 2,814,693 | -3.15 |
| Montgomery | Freeways | 15.20 | 20 | 1,737,983 | 1,846,695 | -5.89 |
| Montgomery | Major Arterials | 28.50 | 32 | 734,499 | 708,744 | 3.63 |
| Montgomery | Minor Arterials | 39.80 | 2 | 15,325 | 12,600 | 21.63 |
| Montgomery | Collectors | 54.67 | 2 | 7,134 | 11,598 | -38.49 |
| Montgomery | Expressways | 23.79 | 5 | 167,883 | 143,435 | 17.04 |
| Montgomery | Ramps | 94.81 | 19 | 63,089 | 91,621 | -31.14 |
| DC | ALL Classes | 57.06 | 626 | 7,280,654 | 6,389,192 | 13.95 |
| DC | Freeways | 16.46 | 4 | 164,143 | 185,300 | -11.42 |
| DC | Major Arterials | 51.17 | 297 | 5,421,992 | 4,307,091 | 25.89 |
| DC | Minor Arterials | 57.40 | 121 | 905,259 | 907,321 | -0.23 |
| DC | Collectors | 93.84 | 195 | 501,548 | 665,182 | -24.60 |
| DC | Expressways | 22.25 | 9 | 287,710 | 324,298 | -11.28 |
| Fairfax | ALL Classes | 40.95 | 198 | 2,195,798 | 2,418,869 | -9.22 |
| Fairfax | Freeways | 16.54 | 8 | 578,959 | 557,809 | 3.79 |
| Fairfax | Major Arterials | 29.63 | 47 | 856,564 | 936,068 | -8.49 |
| Fairfax | Minor Arterials | 42.86 | 56 | 476,054 | 519,270 | -8.32 |
| Fairfax | Collectors | 95.59 | 72 | 223,956 | 306,925 | -27.03 |
| Fairfax | Expressways | 44.89 | 3 | 60,263 | 50,112 | 20.26 |
| Anne Arundel | ALL Classes | 37.82 | 33 | 624,763 | 673,250 | -7.20 |
| Anne Arundel | Freeways | 18.32 | 4 | 140,993 | 122,837 | 14.78 |
| Anne Arundel | Major Arterials | 40.64 | 20 | 356,720 | 435,325 | -18.06 |
| Anne Arundel | Collectors | 96.04 | 4 | 2,329 | 9,500 | -75.48 |
| Anne Arundel | Expressways | 32.88 | 3 | 114,722 | 104,888 | 9.38 |
| Howard | ALL Classes | 25.79 | 25 | 962,657 | 1,028,012 | -6.36 |
| Howard | Freeways | 17.39 | 7 | 583,149 | 604,936 | -3.60 |
| Howard | Major Arterials | 35.00 | 15 | 281,884 | 354,976 | -20.59 |
| Howard | Expressways | 60.97 | 3 | 97,624 | 68,100 | 43.35 |
| St. Mary's | ALL Classes | 40.98 | 15 | 140,373 | 125,551 | 11.81 |
| St. Mary's | Major Arterials | 38.41 | 12 | 137,074 | 116,266 | 17.90 |
| St. Mary's | Minor Arterials | 80.44 | 3 | 3,298 | 9,285 | -64.47 |

Table 7.3 Daily RMSE Statistics

| County | Fclass | Pct RMSE | Num Obs | TotalFlow | TotalCount | Pct Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ALL Counties | ALL Classes | 47.49 | 843 | 3,745,524 | 3,888,632 | -3.68 |
| ALL Counties | Freeways | 23.74 | 113 | 1,636,587 | 1,617,642 | 1.17 |
| ALL Counties | Major Arterials | 61.14 | 326 | 1,201,246 | 1,262,583 | -4.86 |
| ALL Counties | Minor Arterials | 73.20 | 135 | 193,020 | 203,433 | -5.12 |
| ALL Counties | Collectors | 121.41 | 110 | 64,042 | 98,961 | -35.29 |
| ALL Counties | Expressways | 41.58 | 124 | 618,086 | 657,161 | -5.95 |
| ALL Counties | Ramps | 86.40 | 35 | 32,542 | 40,993 | -20.62 |
| PG County | ALL Classes | 38.48 | 332 | 1,758,609 | 1,757,420 | 0.07 |
| PG County | Freeways | 21.25 | 49 | 745,415 | 730,482 | 2.04 |
| PG County | Major Arterials | 53.01 | 122 | 433,928 | 419,206 | 3.51 |
| PG County | Minor Arterials | 59.63 | 40 | 62,281 | 59,316 | 5.00 |
| PG County | Collectors | 122.57 | 9 | 757 | 3,293 | -77.01 |
| PG County | Expressways | 39.84 | 96 | 493,036 | 523,538 | -5.83 |
| PG County | Ramps | 59.33 | 16 | 23,190 | 21,585 | 7.44 |
| Montgomery | ALL Classes | 25.70 | 100 | 712,029 | 704,921 | 1.01 |
| Montgomery | Freeways | 12.99 | 26 | 474,737 | 475,984 | -0.26 |
| Montgomery | Major Arterials | 40.88 | 37 | 191,864 | 179,430 | 6.93 |
| Montgomery | Minor Arterials | 59.83 | 8 | 6,300 | 7,952 | -20.77 |
| Montgomery | Collectors | 126.92 | 4 | 4,422 | 3,243 | 36.37 |
| Montgomery | Expressways | 47.71 | 6 | 25,351 | 18,904 | 34.11 |
| Montgomery | Ramps | 115.86 | 19 | 9,352 | 19,408 | -51.81 |
| Fairfax | ALL Classes | 79.57 | 204 | 515,914 | 550,121 | -6.22 |
| Fairfax | Freeways | 46.46 | 12 | 154,134 | 143,159 | 7.67 |
| Fairfax | Major Arterials | 67.69 | 52 | 207,991 | 210,887 | -1.37 |
| Fairfax | Minor Arterials | 77.51 | 62 | 99,412 | 109,226 | -8.98 |
| Fairfax | Collectors | 120.70 | 75 | 43,228 | 70,114 | -38.34 |
| Fairfax | Expressways | 50.24 | 3 | 11,146 | 8,876 | 25.58 |
| Anne Arundel | ALL Classes | 57.72 | 41 | 168,816 | 255,960 | -34.05 |
| Anne Arundel | Freeways | 45.99 | 6 | 43,199 | 60,730 | -28.87 |
| Anne Arundel | Major Arterials | 65.61 | 22 | 76,461 | 126,052 | -39.34 |
| Anne Arundel | Collectors | 139.40 | 2 | 63 | 850 | -92.58 |
| Anne Arundel | Expressways | 49.40 | 9 | 47,252 | 68,228 | -30.74 |
| Howard | ALL Classes | 32.89 | 32 | 220,004 | 232,157 | -5.23 |
| Howard | Freeways | 22.70 | 7 | 119,546 | 110,728 | 7.96 |
| Howard | Major Arterials | 44.82 | 18 | 68,051 | 89,705 | -24.14 |
| Howard | Collectors | 44.54 | 2 | 3,162 | 3,900 | -18.91 |
| Howard | Expressways | 33.16 | 5 | 29244 | 27824 | 5.10 |
| St. Mary's | ALL Classes | 36.33 | 15 | 15570 | 19095 | -18.46 |
| St. Mary's | Major Arterials | 33.29 | 12 | 15117 | 17593 | -14.07 |
| St. Mary's | Minor Arterials | 86.41 | 3 | 452 | 1502 | -69.85 |

Table 7.4 AM RMSE Statistics

| County | FClass | Pct RMSE | Num Obs | TotalFlow | TotalCount | Pct Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ALL Counties | ALL Classes | 39.03 | 782 | 3,667,168 | 3,669,254 | -0.06 |
| ALL Counties | Freeways | 20.80 | 91 | 1,354,067 | 1,356,892 | -0.21 |
| ALL Counties | Major Arterials | 45.48 | 308 | 1,313,773 | 1,310,055 | 0.28 |
| ALL Counties | Minor Arterials | 59.43 | 135 | 252,800 | 243,517 | 3.81 |
| ALL Counties | Collectors | 102.97 | 104 | 78,850 | 108,989 | -27.65 |
| ALL Counties | Expressways | 30.66 | 108 | 626,710 | 592,130 | 5.84 |
| ALL Counties | Ramps | 70.77 | 36 | 40,966 | 45,471 | -9.91 |
| PG County | ALL Classes | 29.34 | 314 | 1,761,158 | 1,687,670 | 4.35 |
| PG County | Freeways | 14.21 | 41 | 657,477 | 667,116 | -1.44 |
| PG County | Major Arterials | 40.95 | 120 | 498,003 | 452,312 | 10.10 |
| PG County | Minor Arterials | 48.81 | 40 | 81,620 | 68,182 | 19.71 |
| PG County | Collectors | 104.06 | 9 | 1,820 | 4,662 | -60.95 |
| PG County | Expressways | 31.05 | 87 | 495,687 | 471,130 | 5.21 |
| PG County | Ramps | 51.38 | 17 | 26,549 | 24,268 | 9.40 |
| Montgomery | ALL Classes | 29.24 | 89 | 614,647 | 596,844 | 2.98 |
| Montgomery | Freeways | 18.10 | 20 | 349,546 | 342,700 | 2.00 |
| Montgomery | Major Arterials | 36.69 | 32 | 187,257 | 173,585 | 7.88 |
| Montgomery | Minor Arterials | 64.83 | 8 | 10,525 | 8,487 | 24.02 |
| Montgomery | Collectors | 147.02 | 4 | 6,224 | 3,582 | 73.76 |
| Montgomery | Expressways | 12.52 | 6 | 46,677 | 47,287 | -1.29 |
| Montgomery | Ramps | 92.93 | 19 | 14,417 | 21,203 | -32.00 |
| Fairfax | ALL Classes | 55.36 | 195 | 500,386 | 569,683 | -12.16 |
| Fairfax | Freeways | 23.90 | 5 | 80,049 | 77,406 | 3.41 |
| Fairfax | Major Arterials | 45.66 | 51 | 221,986 | 242,809 | -8.58 |
| Fairfax | Minor Arterials | 59.05 | 62 | 124,847 | 135,029 | -7.54 |
| Fairfax | Collectors | 103.17 | 73 | 56,642 | 82,108 | -31.01 |
| Fairfax | Expressways | 49.56 | 4 | 16,860 | 20,131 | -16.25 |
| Anne Arundel | ALL Classes | 58.60 | 37 | 187,000 | 186,729 | 0.15 |
| Anne Arundel | Freeways | 71.48 | 7 | 51,591 | 58,457 | -11.74 |
| Anne Arundel | Major Arterials | 45.53 | 23 | 105,644 | 102,846 | 2.72 |
| Anne Arundel | Collectors | 92.22 | 2 | 1,491 | 1,250 | 19.33 |
| Anne Arundel | Expressways | 24.13 | 3 | 26,116 | 23,951 | 9.04 |
| Howard | ALL Classes | 27.25 | 26 | 215,754 | 237,947 | -9.33 |
| Howard | Freeways | 16.76 | 7 | 119,092 | 122,706 | -2.94 |
| Howard | Major Arterials | 40.06 | 16 | 76,115 | 99,641 | -23.61 |
| Howard | Expressways | 43.73 | 3 | 20,546 | 15,600 | 31.71 |
| St. Mary's | ALL Classes | 49.20 | 16 | 39,554 | 35,211 | 12.34 |
| St. Mary's | Major Arterials | 45.99 | 12 | 38303 | 31511 | 21.56 |
| St. Mary's | Minor Arterials | 77.42 | 4 | 1250 | 3700 | -66.20 |

Table 7.5 PM RMSE Statistics

| County | FClass | Pct RMSE | Num Obs | TotalFlow | TotalCount | Pct Diff |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ALL Counties | ALL Classes | 37.14 | 786 | 10,226,905 | 10,950,521 | -6.61 |
| ALL Counties | Freeways | 22.40 | 104 | 4,540,086 | 4,785,669 | -5.13 |
| ALL Counties | Major Arterials | 39.61 | 300 | 3,072,514 | 3,338,657 | -7.97 |
| ALL Counties | Minor Arterials | 55.15 | 115 | 523,931 | 515,536 | 1.63 |
| ALL Counties | Collectors | 88.99 | 116 | 214,985 | 314,523 | -31.65 |
| ALL Counties | Expressways | 30.34 | 116 | 1,772,918 | 1,852,274 | -4.28 |
| ALL Counties | Ramps | 72.91 | 35 | 102,468 | 115,236 | -11.08 |
| PG County | ALL Classes | 25.92 | 296 | 4,740,900 | 4,899,784 | -3.24 |
| PG County | Freeways | 13.94 | 45 | 2,104,317 | 2,207,070 | -4.66 |
| PG County | Major Arterials | 35.19 | 109 | 1,093,250 | 1,077,031 | 1.51 |
| PG County | Minor Arterials | 60.49 | 28 | 133,223 | 104,875 | 27.03 |
| PG County | Collectors | 87.14 | 8 | 2,790 | 8,575 | -67.46 |
| PG County | Expressways | 28.61 | 90 | 1,344,169 | 1,438,007 | -6.53 |
| PG County | Ramps | 51.19 | 16 | 63,148 | 64,226 | -1.68 |
| Montgomery | ALL Classes | 25.39 | 81 | 1,882,096 | 2,006,790 | -6.21 |
| Montgomery | Freeways | 17.76 | 25 | 1,295,620 | 1,423,749 | -9.00 |
| Montgomery | Major Arterials | 27.48 | 30 | 437,805 | 441,675 | -0.88 |
| Montgomery | Minor Arterials | 97.11 | 2 | 8,730 | 6,550 | 33.29 |
| Montgomery | Expressways | 28.88 | 5 | 100,619 | 83,806 | 20.06 |
| Montgomery | Ramps | 101.10 | 19 | 39,320 | 51,010 | -22.92 |
| Fairfax | ALL Classes | 68.38 | 205 | 1,356,013 | 1,523,007 | -10.96 |
| Fairfax | Freeways | 57.82 | 9 | 381,730 | 330,781 | 15.40 |
| Fairfax | Major Arterials | 31.98 | 51 | 502,859 | 583,834 | -13.87 |
| Fairfax | Minor Arterials | 45.47 | 58 | 283,825 | 314,727 | -9.82 |
| Fairfax | Collectors | 95.28 | 84 | 151,169 | 236,126 | -35.98 |
| Fairfax | Expressways | 46.77 | 3 | 36,427 | 28,913 | 25.99 |
| Anne Arundel | ALL Classes | 43.85 | 44 | 583,636 | 710,833 | -17.89 |
| Anne Arundel | Freeways | 50.97 | 7 | 140,346 | 179,964 | -22.01 |
| Anne Arundel | Major Arterials | 42.39 | 24 | 253,742 | 314,702 | -19.37 |
| Anne Arundel | Collectors | 102.53 | 2 | 692 | 2,400 | -71.16 |
| Anne Arundel | Expressways | 29.82 | 9 | 182,851 | 213,392 | -14.31 |
| Howard | ALL Classes | 27.21 | 29 | 615,427 | 661,047 | -6.90 |
| Howard | Freeways | 18.78 | 7 | 344,510 | 371,502 | -7.27 |
| Howard | Major Arterials | 29.75 | 16 | 192,355 | 226,570 | -15.10 |
| Howard | Collectors | 10.82 | 2 | 9,669 | 10,400 | -7.03 |
| Howard | Expressways | 64.31 | 4 | 68,892 | 52,575 | 31.04 |
| St. Mary's | ALL Classes | 48.33 | 16 | 85690 | 73054 | 17.30 |
| St. Mary's | Major Arterials | 44.75 | 12 | 83653 | 67162 | 24.55 |
| St. Mary's | Minor Arterials | 78.82 | 4 | 2037 | 5892 | -65.43 |

Table 7.6 Off-Peak RMSE Statistics

It is significant that the comparisons between the model estimates and the counts are also fairly good by time period of the day. This supports the validity of the model in a way that a comparison with daily counts does not.

## VMT Comparisons with Traffic Counts and HPMS

The estimated VMT from the model is compared below with that from the links on which there were traffic counts. The fit is good region wide, and especially so in Prince George's County. Additional calibration would be needed to improve the results for Virginia and the District of Columbia.

| Region | Model <br> Daily VMT <br> (thousands) | Count <br> Daily VMT <br> (thousands) | Ratio <br> (Mod/Count) |
| :--- | :---: | :---: | :---: |
| Overall | 15,784 | 15,031 | 1.05 |
| DC | 1,342 | 1,212 | 1.11 |
| VA | 2,279 | 2,512 | 0.91 |
| MD (including PGC) | 11,894 | 11,072 | 1.07 |
| Prince George's County, MD | 5,816 | 5,776 | 1.01 |

Table 7.7 VMT comparison on links that have counts
The total estimated VMT for Prince George's County and the breakdown by functional class from the TransForM model is shown in Table 8.8 below. Two measures of annual VMT are shown. The Annual VMT (365) is obtained by multiplying the daily VMT by 365, whereas the Annual VMT (300) is obtained by multiplying the daily VMT by 300 to account for weekends, when the traffic flows are lower.

Table 7.8 compares the VMT summaries for Prince George's County (by functional class) with the VMT estimates from the HPMS database. It can be seem that the predicted VMT is fairly close to the HPMS estimates. In particular, the total HPMS VMT lies between the two annual VMT estimates calculated. Of course, the HPMS data come from a fairly small sample and may not be accurate.

Table 7.9 compares the VMT totals for all other counties for which the HPMS numbers were available. Again the model numbers were close to the HPMS statistics.

| County | Functional <br> Class | Total Daily <br> VMT (Veh-Mi) | Annual VMT(365) <br> (Millions of Veh-Mi) | Annual VMT (300) <br> (Millions of Veh-Mi) | HPMS Annual VMT <br> (Millions of Veh-Mi) |
| :--- | :--- | :---: | :---: | :---: | :---: |
| PGC | Freeway | $6,668,396$ | 2,433 | 2,000 | 2,665 |
| PGC | Expressway | $4,489,666$ | 1,638 | 1,346 | 1,229 |
| PGC | Maj. Art | $5,932,370$ | 2,165 | 1,779 | 1,917 |
| PGC | Min. Art | $3,524,828$ | 1,286 | 1,057 | 930 |
| PGC | Collector | 663,904 | 242 | 199 | 552 |
| PGC | Ramp | 827,047 | 301 | 248 | NA |
| PGC | Connector | $1,635,873$ | 597 | 490 | 431 |
| PGC | TOTAL | $\mathbf{2 3 , 7 4 2 , 0 8 7}$ | $\mathbf{8 , 6 6 2}$ | $\mathbf{7 , 1 1 9}$ | $\mathbf{7 , 7 2 4}$ |

Table 7.8 VMT Comparison for Prince George's County

| County | Total Daily <br> VMT (Veh-Mi) | Annual VMT(365) <br> (Millions of Veh-Mi) | Annual VMT (300) <br> (Millions of Veh-Mi) | HPMS Annual VMT <br> (Millions of Veh-Mi) |
| :--- | :---: | :---: | :---: | :---: |
| Anne Arundel MD | $12,813,310$ | 4,674 | 3,840 | 5,130 |
| Calvert MD | $1,730,566$ | 629 | 517 | 631 |
| Carroll MD | $4,419,591$ | 1,610 | 1,323 | 1,156 |
| Charles MD | $3,145,026$ | 1,145 | 941 | 1,098 |
| Frederick MD | $7,525,961$ | 2,743 | 2,255 | 2,490 |
| Howard MD | $10,035,546$ | 3,660 | 3,007 | 3,156 |
| Montgomery MD | $19,237,851$ | 7,018 | 5,768 | 6,757 |
| St. Mary's MD | $2,248,703$ | 818 | 673 | 722 |

Table 7.9 VMT Comparison for other Counties (All functional classes)
Here the same comments might apply. While the model estimates are fairly close to the HPMS numbers, it would be helpful if statistical analysis could provide some support for the HPMS measurements and their standard errors.

## Screen Line Analysis

Several screen line analyses were performed as additional verifications of the validity of the model. This analysis was focused upon Prince George's County and was not performed for other jurisdictions.

A map highlighting the various screen lines employed in PG County is shown in Figure 7.6, which follows. Screen line comparisons are based on the aggregation of flows crossing the screen line boundary.

The screenlines examined are:

- Screenline 1A - SE DC/PGC Border- Southern Avenue Boundary
- Screenline 1B - NE DC/PGC Border - Eastern Avenue Boundary
- Screenline 2 - Inside I-95/I-495 Capital Beltway, concentric ring
- Screenline 3 - Mid-County Concentric ring outside the Capital Beltway
- Screenline 4 - Concentric ring along east side of US 301 and west of Patuxent river
- Screenline 5 - Southern Charles/PGC Border, east of MD 210
- Screenline 6 - East side of MD 210
- Screenline 7 - East side of MD 5, from Charles County to the Beltway
- Screenline 8 - South side of MD 4 from Anne Arundel County to the Beltway
- Screenline 9 - South side of MD 214 and MD 332 from DC Border to Anne Arundel County
- Screenline 10 - South side of US 50 from DC Border to Anne Arundel County
- Screenline 11 - East side of Baltimore-Washington Parkway from DC border to Anne Arundel County
- Screenline 12 - Montgomery/PGC border from Howard County to DC


Figure 7.6 Prince George's County Screenlines

## Screenline Results

1. ScreenLine 1A: SE DC/PGC Border- Southern Avenue Boundary

| Name | AB_Flow | BA_Flow | AB_Count | BA_Count | Tot_Flow | Tot_Count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| I-295 | 36,297 |  | 37,501 |  | 36,297 | 37,501 |
| I-295 | 28,235 |  | 37,501 |  | 28,235 | 37,501 |
| MD 210 | 25,588 | 26,070 | 20,100 | 13,300 | 51,658 | 33,400 |
| Owens Rd | 3,729 | 2,854 | 4,750 | 4,750 | 6,583 | 9,500 |
| Wheeler Rd | 6,893 | 8,044 | 10,250 | 10,250 | 14,937 | 20,500 |
| 23rd Pkwy | 9,939 | 10,927 | 4,750 | 4,750 | 20,866 | 9,500 |
| Suitland Pkwy | 20,604 | 20,331 | 25,000 | 25,000 | 40,935 | 50,000 |
| MD 637 (Naylor | 10,003 | 9,884 | 10,250 | 10,250 | 19,887 | 20,500 |
| Rd) | 17,051 | 17,178 | 16,050 | 17,900 | 34,229 | 33,950 |
| MD 5 | 4,250 | 3,543 | 5,000 | 5,000 | 7,793 | 10,000 |
| MD 218 | 16,150 | 16,684 | 14,279 | 14,317 | 32,834 | 28,596 |
| MD 4 | 4,245 | 4,009 | 2,850 | 2,850 | 8,254 | 5,700 |
| Alton St | 2,818 | 2,972 | 6,400 | 7,200 | 5,790 | 13,600 |
| Bowen Rd | 10,547 | 10,841 | 6,750 | 6,750 | 21,388 | 13,500 |
| Benning Rd | 7,476 | 6,753 | 5,000 | 5,000 | 14,229 | 10,000 |
| MD 332 | 16,515 | 17,362 | 13,250 | 13,250 | 33,877 | 26,500 |
| MD 214 | $\mathbf{2 2 0 , 3 4 0}$ | $\mathbf{1 5 7 , 4 5 2}$ | $\mathbf{2 1 9 , 6 8 1}$ | $\mathbf{1 4 0 , 5 6 7}$ | $\mathbf{3 7 7 , 7 9 2}$ | $\mathbf{3 6 0 , 2 4 8}$ |
| Total |  |  |  |  |  |  |

Ratio of Flow versus count $=1.05$
RMSE ( $\mathbf{3 0}$ observations) $=\mathbf{3 5 . 4}$
2. Screenline 1B - NE DC/PGC Border - Eastern Avenue Boundary

| Name | AB_Flow | BA_Flow | AB_Count | BA_Count | Tot_Flow | Tot_Count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| MD 704 | 15,043 | 14,872 | 13,000 | 13,000 | 29,915 | 26,000 |
| Sheriff Rd | 10,011 | 11,279 | 10,500 | 10,500 | 21,290 | 21,000 |
| Addison Rd | 6,011 | 4,827 | 8,500 | 8,500 | 10,838 | 17,000 |
| MD 295 | 54,326 |  | 66,500 |  | 54,326 | 66,500 |
| MD 295 | 55,580 |  | 66,500 |  | 55,580 | 66,500 |
| U.S 50 - EB | 35,324 |  | 31,975 |  | 35,324 | 31,975 |
| U.S 50 - WB |  | 38,194 |  | 31,850 | 38,194 | 31,850 |
| Alt US 1 | 12,561 | 12,802 | 10,000 | 10,000 | 25,363 | 20,000 |
| US 1 | 17,668 | 17,565 | 10,250 | 10,250 | 35,233 | 21,500 |
| MD 5 | 17,051 | 17,178 | 16,050 | 17,900 | 34,229 | 33,950 |
| Sargent Rd | 9,077 | 9,069 | 6,000 | 6,000 | 18,146 | 12,000 |
| MD 212 | 15,061 | 14,658 | 11,600 | 11,300 | 29,719 | 22,900 |
| Chillum Rd | 1,384 | 1,469 | 4,750 | 4,750 | 2,853 | 9,500 |
| MD 650 | 25,730 | 26,884 | 19,000 | 20,000 | 52,614 | 39,000 |
| Total | $\mathbf{2 7 4 , 8 2 7}$ | $\mathbf{1 6 8 , 7 9 7}$ | $\mathbf{2 7 4 , 6 2 5}$ | $\mathbf{1 4 4 , 0 5 0}$ | $\mathbf{4 4 3 , 6 2 4}$ | $\mathbf{4 1 9 , 6 7 5}$ |

Ratio of Flow versus count $=\mathbf{1 . 0 6}$

RMSE ( 24 observations) $=29.9$
3. Screenline 2 - Inside I-95/I-495 Capital Beltway, concentric ring

| Name | ab_flow | ba_flow | ab_count | ba_count | tot_flow | tot_count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| MD 650 | 39,873 | 40,064 | 36,500 | 36,500 | 79,937 | 73,000 |
| MD 212 | 14,196 | 13,506 | 10,000 | 10,000 | 27,702 | 20,000 |
| Cherry Hill Rd | 1,624 | 10,899 | 11,500 | 11,500 | 21,523 | 23,000 |
| US 1 | 23,999 | 23,297 | 27,600 | 29,200 | 47,296 | 56,800 |
| Rhode Island Ave | 7,484 | 6,968 | 8,500 | 8,500 | 14,452 | 17,000 |
| Cherrywood Lane | 6,787 | 6,312 | 6,250 | 6,250 | 13,099 | 12,500 |
| MD 201 NB | 22,700 |  | 25,700 |  | 22,700 | 25,700 |
| MD 201 NB | 22,705 |  | 27,200 |  | 22,705 | 27,200 |
| MD 193 | 25,114 | 24,310 | 25,000 | 25,000 | 49,424 | 50,000 |
| BW Pkwy NB | 27,425 |  | 47,500 |  | 27,425 | 47,500 |
| BW Pkwy NB |  | 29,699 |  | 47,500 | 29,699 | 47,500 |
| Good Luck Rd | 6,444 | 6,720 | 8,750 | 8,750 | 13,164 | 17,500 |
| MD 450 | 19,934 | 18,482 | 22,250 | 22,250 | 38,416 | 45,000 |
| MD 950 (Garden | 142 |  | 3,000 |  |  |  |
| City) | 36,955 |  | 48,000 |  | 36,955 | 48,000 |
| US 50 EB |  | 38,460 |  | 39,200 | 38,460 | 39,200 |
| US 50 EB | 18,459 | 17,657 | 15,000 | 15,000 | 36,116 | 30,000 |
| MD 704 | 3,551 | 2,652 | 4,750 | 4,750 | 6,203 | 9,500 |
| Ardwick Ardmore Rd | 30,256 | 26,855 | 30,000 | 30,000 | 57,111 | 60,000 |
| MD 202 | 3,678 | 3,331 | 4,500 | 4,500 | 7,009 | 9,000 |
| Arena Dr | 34,984 | 36,864 | 36,500 | 36,500 | 71,848 | 73,000 |
| MD 214 | 8,303 | 7,604 | 6,500 | 6,500 | 15,907 | 13,000 |
| Ritchie-Marlboro Rd | 1,510 | 777 | 2,750 | 2,750 | 2,287 | 5,500 |
| Darcy Rd | 26,725 | 30,048 | 32,000 | 32,000 | 56,773 | 64,000 |
| MD 4 |  | 10,000 |  | 16,867 | 10,000 |  |
| Suitland Pkwy EB | 16,867 |  | 10,000 |  | 8,917 | 10,000 |
| Suitland Pkwy EB | 8,917 |  | 7,750 | 7,750 | 14,749 | 15,500 |
| Forestville Rd | 5,267 | 9,482 | 70,781 | 21,000 |  |  |
| Suitland Rd | 10,984 | 10,797 | 10,050 | 10,050 | 21,781 | 4,250 |
| Auth Rd | 3,101 | 1,673 | 2,125 | 2,125 | 4,774 |  |
| MD 5 | 33,363 | 34,092 | 32,500 | 32,500 | 67,455 | 65,000 |
| Temple Hill Rd | 53,313 |  | 67,200 |  | 53,313 | 67,200 |
| MD 414 NB |  | 25,276 |  | 24,000 | 25,276 | 24,000 |
| MD 414 NB | 23,976 |  | 24,000 |  | 23,976 | 24,000 |
| Livingston Rd | 11,727 | 12,142 | 8,000 | 8,000 | 23,869 | 16,000 |
| MD 210 | 21,038 | 21,075 | 12,500 | 12,500 | 42,113 | 25,000 |
| I-295 | 28,235 |  | 37,501 |  | 28,235 | 37,501 |
| I-295 | 36,297 |  | 37,501 |  | 36,297 | 37,501 |
| Total | 644,933 | 459,042 | $\mathbf{6 9 8 , 8 7 7}$ | $\mathbf{4 7 3 , 5 7 5}$ | $\mathbf{1 , 1 0 3 , 9 7 5}$ | $\mathbf{1 , 1 7 3 , 8 5 2}$ |
|  |  |  |  |  |  |  |

Ratio of Flow versus count $=0.94$
RMSE ( 60 observations) $=26.8$
4. Screenline 3 - Mid-County Concentric ring outside the Capital Beltway

| Name | ab_flow | ba_flow | ab_count | ba_count | tot_flow | tot_count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Oxon Hill Rd | 3,468 | 3,922 | 5,500 | 5,500 | 7,390 | 11,000 |
| Livingston Rd W | 3,572 | 1,924 | 6,500 | 6,500 | 5,496 | 13,000 |
| SR 210 | 45,110 | 43,559 | 25,000 | 24,000 | 88,669 | 49,000 |
| Allentown Rd | 7,218 | 7,147 | 11,000 | 11,000 | 14,365 | 22,000 |
| Temple Hill Rd | 3,035 | 3,283 | 4,500 | 4,500 | 6,318 | 9,000 |
| Old Branch Ave | 7,937 | 8,058 | 8,500 | 8,500 | 15,995 | 17,000 |
| MD 5 | 32,862 | 35,216 | 45,501 | 45,501 | 68,078 | 91,001 |
| MD 223 | 4,629 | 4,885 | 10,250 | 10,250 | 9,514 | 20,500 |
| Dangerfield Rd | 1,604 | 1,563 | 3,500 | 3,500 | 3,167 | 7,000 |
| Rosaryville Rd | 8,740 | 9,450 | 9,250 | 9,250 | 18,190 | 18,500 |
| S. Osborne Rd | 6,917 | 7,629 | 5,500 | 5,500 | 14,546 | 11,000 |
| MD 4 EB | 16,095 |  | 24,000 |  | 16,095 | 24,000 |
| MD 4 EB |  | 16,863 |  | 25,779 | 16,863 | 25,779 |
| Marlboro Pk | 8,254 | 7,647 | 5,500 | 5,500 | 15,901 | 11,000 |
| Old Marlboro Pk | 987 | 1,075 | 2,500 | 2,500 | 2,062 | 5,000 |
| Ritchie Marlboro Rd | 5,184 | 4,815 | 3,250 | 3,250 | 9,999 | 6,500 |
| Brown Station Rd | 1,798 | 2,117 | 3,250 | 3,250 | 3,915 | 6,500 |
| MD 202 | 15,860 | 16,587 | 13,326 | 13,326 | 32,447 | 26,652 |
| Oak Grove Rd | 5,285 | 4,780 | 2,250 | 2,250 | 10,065 | 4,500 |
| MD 214 | 18,528 | 19,622 | 15,000 | 15,000 | 38,150 | 30,000 |
| Woodmore Rd | 4,442 | 3,727 | 5,000 | 5,000 | 8,169 | 10,000 |
| US 50 EB | 59,669 |  | 53,000 |  | 59,669 | 53,000 |
| US 50 EB | 60,873 |  | 53,000 |  | 60,873 | 53,000 |
| MD 450 | 11,512 | 11,908 | 11,500 | 11,500 | 23,420 | 23,000 |
| Daisy Ln | 7,373 | 7,092 | 2,500 | 2,500 | 14,465 | 5,000 |
| Prospect Hill Rd | 2,656 | 2,096 | 3,500 | 3,500 | 4,752 | 7,000 |
| MD 564 | 7,618 | 7,808 | 5,500 | 5,500 | 15,426 | 11,000 |
| Good Luck Rd | 24 | 108 | 1,750 | 1,750 | 132 | 3,500 |
| Soil Conser. Rd | 4,232 | 5,461 | 7,500 | 7,500 | 9,693 | 15,000 |
| BW Parkway NB | 31,289 |  | 43,400 |  | 31,289 | 43,400 |
| BW Parkway NB |  | 29,530 |  | 43,000 | 29,530 | 43,000 |
| Odell Rd | 1,123 | 1,108 | 2,500 | 2,500 | 2,231 | 5,000 |
| Old Baltimore Rd | 4,443 | 4,487 | 5,500 | 5,500 | 8,930 | 11,000 |
| US 1 | 15,880 | 18,249 | 22,500 | 22,500 | 34,129 | 45,000 |
| Virginia Manor Rd | 7,900 | 7,384 | 4,000 | 4,000 | 15,284 | 8,000 |
| I-95 NB | 84,623 |  | 77,500 |  | 84,623 | 77,500 |
| I-95 NB | 85,047 |  | 77,500 |  | 85,047 | 77,500 |
| Old Gunpowder Rd | 10,613 | 9,507 | 4,500 | 4,500 | 20,120 | 9,000 |
| Total | 596,400 | 308,607 | 584,727 | 324,106 | 905,007 | 908,832 |
|  |  |  |  |  |  |  |

Ratio of Flow versus count $=\mathbf{1 . 0 0}$
RMSE ( 68 observations) $=43.0$
5. Screenline 4 - Concentric ring along east side of US 301 and west of Patuxent river

| Name | ab_flow | ba_flow | ab_count | ba_count | tot_flow | tot_count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| I-95 NB | 87,306 |  | 95,000 |  | 87,306 | 95,000 |
| I-95 NB | 86,598 |  | 94,000 |  | 86,598 | 94,000 |
| MD 216 | 8,407 | 8,064 | 11,000 | 11,000 | 16,471 | 22,000 |
| US 1 NB |  | 18,517 |  | 17,500 | 18,517 | 17,500 |
| US 1 NB | 17,663 |  | 17,500 |  | 17,663 | 17,500 |
| MD 198 | 18,660 | 18,188 | 20,000 | 20,000 | 36,848 | 40,000 |
| Montpelier Dr | 1,823 | 424 | 2,250 | 2,250 | 2,247 | 4,500 |
| BW Parkway - NB | 36,832 |  | 42,000 |  | 36,832 | 42,000 |
| BW Parkway - NB | 35,225 |  | 43,000 |  | 35,225 | 43,000 |
| MD 3 - NB |  | 32,048 |  | 37,500 | 32,048 | 37,500 |
| MD 3 - NB | 30,345 |  | 37,500 |  | 30,345 | 37,500 |
| US 50 | 43,994 |  | 37,700 |  | 43,994 | 37,700 |
| US 50 | 45,826 |  | 39,567 |  | 45,826 | 39,567 |
| Governor Bridge Rd | 44 | 39 | 2,500 | 2,500 | 83 | 5,000 |
| MD 214 | 11,155 | 12,398 | 6,000 | 6,000 | 23,553 | 12,000 |
| Marlboro Pk | 2,585 | 1,300 | 4,000 | 4,000 | 3,885 | 8,000 |
| MD 4 - EB |  | 24,415 |  | 24,100 | 24,415 | 24,100 |
| MD 4 - EB | 28,359 |  | 23,700 |  | 28,359 | 23,700 |
| Croom Station Rd | 2,382 | 1,965 | 2,000 | 2,000 | 4,347 | 4,000 |
| MD 382 | 5,284 | 4,648 | 1,750 | 1,750 | 9,932 | 3,500 |
| Heathermore Blvd | 3,911 | 3,966 | 4,500 | 4,500 | 7,877 | 9,000 |
| Trumps Hill Rd | 927 | 977 | 250 | 250 | 1,904 | 500 |
| Fairhaven Ave | 1,492 | 1,472 | 3,000 | 3,000 | 2,964 | 6,000 |
| Rosaryville Rd | 1,399 | 1,409 | 2,500 | 2,500 | 2,808 | 5,000 |
| Frank Tippett Rd | 151 | 491 | 500 | 500 | 642 | 1,000 |
| Dyson Rd | 720 | 584 | 1,250 | 1,250 | 1,304 | 2,500 |
| Missouri Ave | 58 | 67 | 1,250 | 1,250 | 125 | 2,500 |
| MD 381 | 5,236 | 5,828 | 4,500 | 4,500 | 11,064 | 9,000 |
| Cedarville Rd | 2,180 | 2,172 | 2,000 | 2,000 | 4,352 | 4,000 |
| Total | $\mathbf{4 7 8 , 5 6 2}$ | $\mathbf{1 3 8 , 9 7 2}$ | $\mathbf{4 9 9 , 2 1 7}$ | $\mathbf{1 4 8 , 3 5 0}$ | $\mathbf{6 1 7 , 5 3 4}$ | $\mathbf{6 4 7 , 5 6 7}$ |

Ratio of Flow versus count $=0.95$
RMSE (48 observations) = 24.9
6. Screenline 5 - Southern Charles/PGC Border, east of MD 210

| Name | ab_flow | ba_flow | ab_count | ba_count | tot_flow | tot_count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| MD 381 | 5,241 | 4,314 | 2,500 | 2,500 | 9,555 | 5,000 |
| Doctor Bowen Rd | 257 | 262 | 500 | 500 | 519 | 1,000 |
| MD 382 | 361 | 323 | 500 | 500 | 684 | 1,000 |
| US 301/MD 5 - NB |  | 53,766 |  | 43,500 | 53,766 | 43,500 |
| US 301/MD 5 - NB | 52,970 |  | 43,500 |  | 52,970 | 43,500 |
| MD 210 | 9,246 | 8,850 | 15,000 | 15,000 | 18,096 | 30,000 |
| Total | $\mathbf{6 8 , 0 7 5}$ | $\mathbf{6 7 , 5 1 5}$ | $\mathbf{6 2 , 0 0 0}$ | $\mathbf{6 2 , 0 0 0}$ | $\mathbf{1 3 5 , 5 9 0}$ | $\mathbf{1 2 4 , 0 0 0}$ |

Ratio of Flow versus count $=1.09$
RMSE (10 observations) = $\mathbf{1 5 2 . 0}$
7. Screenline 6 - East side of MD 210

| Name | AB_Flow | BA_Flow | AB_Count | BA_Count | Tot_Flow | Tot_Count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| MD 228 (Berry | 17,556 | 16,296 | 15,500 | 15,500 | 33,852 | 31,000 |
| Rd) | 1,343 | 1,613 | 4,750 | 4,750 | 2,956 | 9,500 |
| MD 373 | 2,844 | 2,878 | 1,750 | 1,750 | 5,722 | 3,500 |
| Farmington Rd | 2,128 | 2,135 | 3,000 | 3,000 | 4,263 | 6,000 |
| Old Fort Rd S | 8,018 | 7,785 | 4,250 | 4,250 | 15,803 | 8,500 |
| Livington Rd S | 9,388 | 9,798 | 6,500 | 6,500 | 19,186 | 13,000 |
| Old Fort Rd N | 4,277 | 5,150 | 7,000 | 7,000 | 9,427 | 14,000 |
| Palmer Rd | 10,813 | 11,145 | 4,250 | 4,250 | 21,958 | 8,500 |
| Livington Rd N | 13,600 | 15,804 | 15,250 | 15,250 | 29,404 | 30,500 |
| MD 414 | 17,546 |  | 19,500 |  | 17,546 | 19,500 |
| Ramp to I-95/I- |  |  |  |  |  |  |
| 495 | 59,771 |  | 72,000 |  | 59,771 | 72,000 |
| I-95/I-495 | 56,608 |  | 66,400 |  | 56,608 | 66,400 |
| I-95/I-495 | 8,435 | 7,810 | 16,000 | 8,000 | 16,245 | 8,000 |
| Livington Rd | 11,257 | 10,242 | 14,000 | 7,000 | 21,499 | 7,000 |
| Southern Ave | $\mathbf{2 2 3 , 5 8 4}$ | $\mathbf{9 0 , 6 5 6}$ | $\mathbf{2 5 0 , 1 5 0}$ | $\mathbf{7 7 , 2 5 0}$ | $\mathbf{3 1 4 , 2 4 0}$ | $\mathbf{2 9 7 , 4 0 0}$ |
| Total |  |  |  |  |  |  |

Ratio of Flow versus count $=1.06$
RMSE ( $\mathbf{2 5}$ observations) $=\mathbf{3 8 . 6}$
8. Screenline 7 - East side of MD 5, from Charles County to the Beltway

| Name | AB_Flow | BA_Flow | AB_Count | BA_Count | Tot_Flow | Tot_Count |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| US 301 | 14,646 |  | 15,500 |  | 14,646 | 15,500 |
| US 301 | 14,779 |  | 15,500 |  | 14,779 | 15,500 |
| MD 373 | 1,795 | 1,346 | 1,500 | 1,500 | 3,141 | 3,000 |
| Brandywine Rd | 5,266 | 5,084 | 4,900 | 4,100 | 10,350 | 9,000 |
| Surratts Rd | 6,414 | 6,293 | 6,000 | 6,000 | 12,707 | 12,000 |
| MD 223 | 5,435 | 5,689 | 10,613 | 10,613 | 11,124 | 10,613 |
| Malcolm Rd | 3,177 | 3,145 | 3,750 | 3,750 | 6,322 | 7,500 |
| Coventry Way | 2,002 | 2,229 | 12,500 | 12,500 | 4,231 | 25,000 |
| Old Alexandria Ferry Rd | 7,494 | 8,119 | 1,500 | 8,500 | 15,613 | 10,000 |
| MD 337 | 20,326 | 18,300 | 15,000 | 15,000 | 38,626 | 30,000 |
| I-95/l-495 | 66,789 |  | 84,000 |  | 66,789 | 84,000 |
| \|-95/l-495 | 60,999 |  | 84,000 |  | 60,999 | 84,000 |
| Total | 209,122 | 50,205 | 254,763 | 61,963 | 259,327 | 306,113 |

Ratio of Flow versus count $=0.85$
RMSE ( $\mathbf{2 0}$ observations) $=51.3$
9. Screenline 8 - South side of MD 4 from Anne Arundel County to the Beltway

| Name | AB_Flow | BA_Flow | AB_Count | BA_Count | Tot_Flow | Tot_Count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| I-95/l-495 | 70,285 |  | 81,800 |  | 70,285 | 81,800 |
| I-95/l-495 | 77,370 |  | 85,800 |  | 77,370 | 85,800 |
| Old Marlboro Pk | 2,651 | 3,079 | 6,500 | 6,500 | 5,730 | 13,000 |
| MD 337 | 11,033 | 7,059 | 10,250 | 10,250 | 18,092 | 21,500 |
| Dower House Rd | 9,449 | 9,225 | 6,000 | 6,000 | 18,674 | 12,000 |
| MD 223 | 11,729 | 11,971 | 8,995 | 8,995 | 23,700 | 17,990 |
| Old Crain Hwy | 6,502 | 2,345 | 6,500 | 2,500 | 8,847 | 9,000 |
| U.S. 301 | 12,866 |  | 17,000 |  | 12,866 | 17,000 |
| U.S. 301 | 12,371 |  | 16,300 |  | 12,371 | 16,300 |
| Total | $\mathbf{2 1 4 , 2 5 6}$ | $\mathbf{3 3 , 6 7 9}$ | $\mathbf{2 3 9 , 1 4 5}$ | $\mathbf{3 4 , 2 4 5}$ | $\mathbf{2 4 7 , 9 3 5}$ | $\mathbf{2 7 4 , 3 9 0}$ |

Ratio of Flow versus count $=\mathbf{0 . 9 0}$
RMSE ( $\mathbf{1 4}$ observations) $=\mathbf{2 5 . 0}$
10. ScreenLine 9: South side of MD 214 and MD 332 from DC Border to Anne Arundel County

| Name | AB_Flow | BA_Flow | AB_Count | BA_Count | Tot_Flow | Tot_Count |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Larchmont Ave | 3,650 | 2,921 | 4,500 | 4,500 | 6,571 | 9,000 |
| Suffolk Ave | 2,025 | 2,094 | 2,500 | 2,500 | 4,119 | 5,000 |
| Rollins Ave | 169 | 199 | 1,500 | 1,500 | 368 | 3,000 |
| Addison Rd | 14,385 | 14,652 | 11,700 | 12,400 | 29,037 | 24,100 |
| Shady Glen Dr | 9,422 | 9,666 | 4,000 | 4,000 | 19,088 | 8,000 |
| Ritchie Rd | 12,018 | 13,969 | 13,250 | 14,850 | 25,987 | 28,100 |
| Hampton Park Blvd | 15,873 | 17,525 | 13,650 | 17,050 | 33,398 | 30,700 |
| I-95/I-495 | 76,289 |  | 84,900 |  | 76,289 | 84,900 |
| \|-95/|-495 | 83,292 |  | 84,600 |  | 83,292 | 84,600 |
| Harry S Truman Dr | 8,841 | 9,071 | 9,000 | 9,000 | 17,912 | 18,000 |
| MD 202 | 23,337 | 19,647 | 21,300 | 19,300 | 42,984 | 40,600 |
| Campus Way | 1,963 | 2,977 | 9,850 | 12,450 | 4,940 | 22,300 |
| Kettering Dr | 4,863 | 4,468 | 4,300 | 4,500 | 9,331 | 8,800 |
| MD 193 | 8,643 | 9,530 | 6,600 | 6,600 | 18,173 | 13,200 |
| Church Rd | 6,120 | 6,004 | 2,000 | 2,000 | 12,124 | 4,000 |
| Jennings Mill Dr | 675 | 702 | 1,850 | 1,900 | 1,377 | 3,750 |
| US 301 | 16,125 |  | 25,200 |  | 16,125 | 25,200 |
| US 301 | 16,213 |  | 24,700 |  | 16,213 | 24,700 |
| Total | 303,903 | 113,425 | 325,400 | 112,550 | 417,328 | 437,950 |

Ratio of Flow versus count $=\mathbf{0 . 9 5}$
RMSE ( $\mathbf{3 2}$ observations) $=\mathbf{3 0 . 0}$
11. ScreenLine 10: South side of US 50 from DC Border to Anne Arundel County

| Name | AB Flow | BA Flow | AB_Count | BA_Count | Tot Flow | Tot Count |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| MD 295 | 55,580 |  | 66,500 |  | 55,580 | 66,500 |
| MD 295 | 54,326 |  | 66,500 |  | 54,326 | 66,500 |
| Columbia Park Rd | 13,175 | 13,561 | 9,450 | 7,900 | 26,736 | 17,350 |
| MD 202 | 25,184 | 25,026 | 24,325 | 25,100 | 50,210 | 49,425 |
| MD 410 | 10,065 | 7,040 | 9,700 | 11,500 | 17,105 | 21,200 |
| I-95/l-495 | 92,440 |  | 96,000 |  | 92,440 | 96,000 |
| I-95/l-495 | 99,906 |  | 92,000 |  | 99,906 | 92,000 |
| Whitfield Chapel Rd | 6,201 | 7,627 | 6,500 | 6,500 | 13,828 | 13,000 |
| MD 704 | 14,097 | 11,073 | 14,000 | 14,000 | 25,170 | 28,000 |
| Lottsford Vista Rd | 3,622 | 3,256 | 4,500 | 4,500 | 6,878 | 9,000 |
| MD 193 | 10,122 | 9,850 | 8,500 | 8,500 | 19,972 | 17,000 |
| Church Rd | 1,521 | 2,065 | 2,000 | 2,000 | 3,586 | 4,000 |
| MD 197 | 11,622 | 11,953 | 23,500 | 23,500 | 23,575 | 47,000 |
| US 301 | 22,731 |  | 32,000 |  | 22,731 | 32,000 |
| US 301 | 23,761 |  | 29,600 |  | 23,761 | 29,600 |
| Total | 444,353 | 91,451 | 485,075 | 103,500 | 535,804 | 588,575 |

Ratio of Flow versus count $=0.91$
RMSE ( 24 observations) $=\mathbf{2 4 . 4}$
12. Screenline 11 - East side of Baltimore Washington Parkway from DC border to Anne Arundel County

| Name | AB_Flow | BA_Flow | AB_Count | BA_Count | Tot_Flow | Tot_Count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Eastern Ave | 14,057 | 11,626 | 7,750 | 7,750 | 25,683 | 15,500 |
| US 50 | 25,566 |  | 44,361 |  | 25,566 | 44,361 |
| US 50 | 23,049 |  | 36,000 |  | 23,049 | 36,000 |
| MD 202 | 33,211 | 26,413 | 24,054 | 21,200 | 59,624 | 45,254 |
| MD 450 | 24,944 | 25,930 | 17,850 | 17,850 | 50,874 | 39,500 |
| MD 410 | 23,872 | 24,103 | 16,000 | 16,000 | 47,975 | 32,000 |
| Good Luck Rd | 9,864 | 10,666 | 8,500 | 8,500 | 20,530 | 17,000 |
| I-95/l-495 | 99,534 |  | 105,300 |  | 99,534 | 105,300 |
| I-95/l-495 | 93,784 |  | 94,300 |  | 93,784 | 94,300 |
| MD 193 | 28,128 | 26,531 | 18,000 | 18,400 | 54,659 | 36,400 |
| Beaver Dam | 2,599 | 3,146 | 500 | 500 | 5,745 | 1,000 |
| Rd | Powder Mill Rd | 11,826 | 10,779 | 9,000 | 9,000 | 22,605 |
| MD 197 | $\mathbf{1 7 , 6 2 6}$ | 17,843 | $\mathbf{1 8 , 5 0 0}$ | 18,500 | $\mathbf{3 5 , 4 6 9}$ | $\mathbf{3 7 , 0 0 0}$ |
| Total | $\mathbf{4 0 8 , 0 6 0}$ | $\mathbf{1 5 7 , 0 3 7}$ | $\mathbf{4 0 0 , 1 1 5}$ | $\mathbf{1 1 7 , 7 0 0}$ | $\mathbf{5 6 5 , 0 9 7}$ | $\mathbf{5 2 1 , 6 1 5}$ |

Ratio of Flow versus count $=1.08$
RMSE ( $\mathbf{2 2}$ observations) = 31.4
13. Screenline $\mathbf{1 2}$ - Montgomery/PGC border from Howard County to DC

| Name | AB_Flow | BA_Flow | AB_Count | BA_Count | Tot_Flow | Tot_Count |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Eastern Ave | 9,732 | 9,334 | 7,250 | 7,250 | 19,066 | 14,500 |
| Sheridan Rd | 1,059 | 1,319 | 2,925 | 2,975 | 2,378 | 5,900 |
| Ray Rd | 466 | 573 | 1,500 | 1,500 | 1,039 | 3,000 |
| MD 410 | 16,540 | 16,371 | 12,000 | 12,000 | 32,911 | 24,000 |
| MD 193 | 24,689 | 24,547 | 18,825 | 20,000 | 49,236 | 38,825 |
| Merrimac Dr | 2,021 | 2,025 | 2,225 | 2,225 | 4,046 | 4,500 |
| Metzerott Rd | 12,968 | 13,007 | 4,225 | 5,200 | 25,975 | 9,425 |
| Adephi Rd | 12,195 | 11,860 | 11,750 | 11,750 | 24,055 | 23,500 |
| I-95/l-495 | 124,315 |  | 113,900 |  | 124,315 | 113,900 |
| I-95/l-495 | 132,283 |  | 111,400 |  | 132,283 | 111,400 |
| Powder Mill Rd | 14,868 | 14,117 | 9,000 | 9,000 | 28,985 | 18,000 |
| Cherry Hill Rd | 12,000 | 12,838 | 14,000 | 14,000 | 24,838 | 28,000 |
| Calverton Rd | 11,556 | 10,039 | 7,750 | 7,750 | 21,595 | 15,500 |
| Briggs Ch Rd | 9,723 | 11,062 | 8,000 | 8,000 | 20,785 | 16,000 |
| Greencastle Rd | 8,349 | 8,891 | 2,500 | 2,500 | 17,240 | 5,000 |
| MD 198 | 29,873 |  | 22,500 |  | 29,873 | 22,500 |
| MD 198 | 29,887 |  | 22,500 |  | 29,887 | 22,500 |
| Brooklyn Br Rd | 1,454 | 704 | 2,500 | 1,500 | 2,158 | 4,000 |
| Total | $\mathbf{4 5 3 , 9 7 8}$ | $\mathbf{1 3 6 , 6 8 7}$ | $\mathbf{3 7 4 , 7 5 0}$ | $\mathbf{1 0 5 , 6 5 0}$ | $\mathbf{5 9 0}, 665$ | $\mathbf{4 8 0 , 4 5 0}$ |

## Ratio of Flow versus count $=1.23$ <br> RMSE ( $\mathbf{3 2}$ observations) $=\mathbf{3 9 . 3}$

## Conclusion

The results presented in this chapter indicate that the model appears to achieve its principal objectives of generating traffic flows in Prince George's County that are close to those observed and also does a credible job of matching flows elsewhere in the region. Further calibration of the model is certainly possible and is warranted on an ongoing basis and especially when additional and higher quality traffic counts become available. Also, the calibration should be revisited when new survey data are obtained.

